

Hilti HIT-HY 150 MAX with HIT-TZ

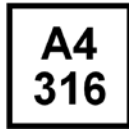
Injection mortar system	Benefits
 <p>Hilti HIT-HY 150 MAX 330 ml foil pack</p> <p>(also available as 500 ml and 1400 ml foil pack)</p>  <p>Statik mixer</p>  <p>HIT-TZ HIT-RTZ rod</p>	<ul style="list-style-type: none"> - suitable for cracked and non-cracked concrete C 20/25 to C 50/60 - hammer drilled and diamond cored bore holes - high loading capacity - suitable for dry and water saturated concrete - under water application - No cleaning required



Concrete



Tensile zone



Corrosion resistance



European Technical Approval



CE conformity



PROFIS Anchor design software

Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval ^{a)}	DIBt, Berlin	ETA-04/0084 / 2009-12-09

a) All data given in this section according ETA-04/0084, issue 2009-12-09.

Basic loading data (for a single anchor)

All data in this section applies to

For details see Simplified design method

- Correct setting (See setting instruction)
- No edge distance and spacing influence
- Steel failure
- Base material thickness, as specified in the table
- Embedment depth, as specified in the table
- One anchor material, as specified in the tables
- Concrete C 20/25, $f_{ck,cube} = 25 \text{ N/mm}^2$
- Temperate range I
(min. base material temperature -40°C , max. long term/short term base material temperature: $+50^\circ\text{C}/80^\circ\text{C}$)
- Installation temperature range $+5^\circ\text{C}$ to $+40^\circ\text{C}$

Embedment depth and base material thickness for the basic loading data.

Mean ultimate resistance, characteristic resistance, design resistance, recommended loads.

Anchor size	M8	M10	M12	M16	M20
Embedment depth [mm]	55	65	75	90	120
Base material thickness [mm]	110	130	150	180	240

Mean ultimate resistance ^{a)}: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$, anchor HIT-TZ

Anchor size	M8	M10	M12	M16	M20
Non-cracked concrete					
Tensile $N_{Ru,m}$ HIT-TZ [kN]	21,3	26,7	33,3	57,5	88,5
Shear $V_{Ru,m}$ HIT-TZ [kN]	11,6	17,9	26,3	49,4	77,7
Cracked concrete					
Tensile $N_{Ru,m}$ HIT-TZ [kN]	12,0	21,3	26,7	40,0	53,3
Shear $V_{Ru,m}$ HIT-TZ [kN]	12,0	17,9	26,3	49,4	77,7

Characteristic resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$, anchor HIT-TZ

Anchor size	M8	M10	M12	M16	M20
Non-cracked concrete					
Tensile N_{Rk} HIT-TZ [kN]	16,0	20,0	25,0	43,1	66,4
Shear V_{Rk} HIT-TZ [kN]	11,0	17,0	25,0	47,0	74,0
Cracked concrete					
Tensile N_{Rk} HIT-TZ [kN]	9,0	16,0	20,0	30,0	40,0
Shear V_{Rk} HIT-TZ [kN]	9,0	17,0	25,0	47,0	74,0

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$, anchor HIT-TZ

Anchor size	M8	M10	M12	M16	M20
Non-cracked concrete					
Tensile N_{Rd} HIT-TZ [kN]	10,7	13,3	16,7	28,7	44,3
Shear V_{Rd} HIT-TZ [kN]	8,8	13,6	20,0	37,6	59,2
Cracked concrete					
Tensile N_{Rd} HIT-TZ [kN]	6,0	10,7	13,3	20,0	26,7
Shear V_{Rd} HIT-TZ [kN]	6,0	13,6	20,0	37,6	53,3

Recommended loads ^{a)}: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$, anchor HIT-TZ

Anchor size	M8	M10	M12	M16	M20
Non-cracked concrete					
Tensile N_{rec} HIT-TZ [kN]	7,6	9,5	11,9	20,5	31,6
Shear V_{rec} HIT-TZ [kN]	6,3	9,7	14,3	26,9	42,3
Cracked concrete					
Tensile N_{rec} HIT-TZ [kN]	4,3	7,6	9,5	14,3	19,0
Shear V_{rec} HIT-TZ [kN]	4,3	9,7	14,3	26,9	38,1

a) With overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Service temperature range

Hilti HIT-HY 150 MAX injection mortar with anchor rod HIT-TZ may be applied in the temperature ranges given below. An elevated base material temperature may lead to a reduction of the design bond resistance.

Temperature range	Base material temperature	Maximum long term base material temperature	Maximum short term base material temperature
Temperature range I	-40 °C to +80 °C	+50 °C	+80 °C

Max short term base material temperature

Short-term elevated base material temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Max long term base material temperature

Long-term elevated base material temperatures are roughly constant over significant periods of time.

Materials

Mechanical properties of HIT-(R)TZ

Anchor size			M8	M10	M12	M16	M20
Nominal tensile strength f_{uk}	HIT-TZ	[N/mm ²]	600	600	600	600	600
	HIT-RTZ	[N/mm ²]	600	600	600	600	600
Yield strength f_{yk}	HIT-TZ	[N/mm ²]	480	480	480	480	480
	HIT-RTZ	[N/mm ²]	480	480	480	480	480
Stressed cross-section A_s	HIT-TZ	[mm ²]	36,6	58,0	84,3	157	245
Moment of resistance W	HIT-TZ	[mm ³]	31,9	62,5	109,7	278	542

Material quality

Part	Material
HIT-TZ	C-steel cold formed steel galvanized $\geq 5\mu\text{m}$
HIT-RTZ	stainless steel cold formed 1.4404 and 1.4401

Anchor dimensions

Anchor size	M8	M10	M12	M16	M20
HIT-(R)TZ	M8x55	M10x65	M12x75	M16x90	M20x120
Anchor embedment depth [mm]	55	65	75	90	120

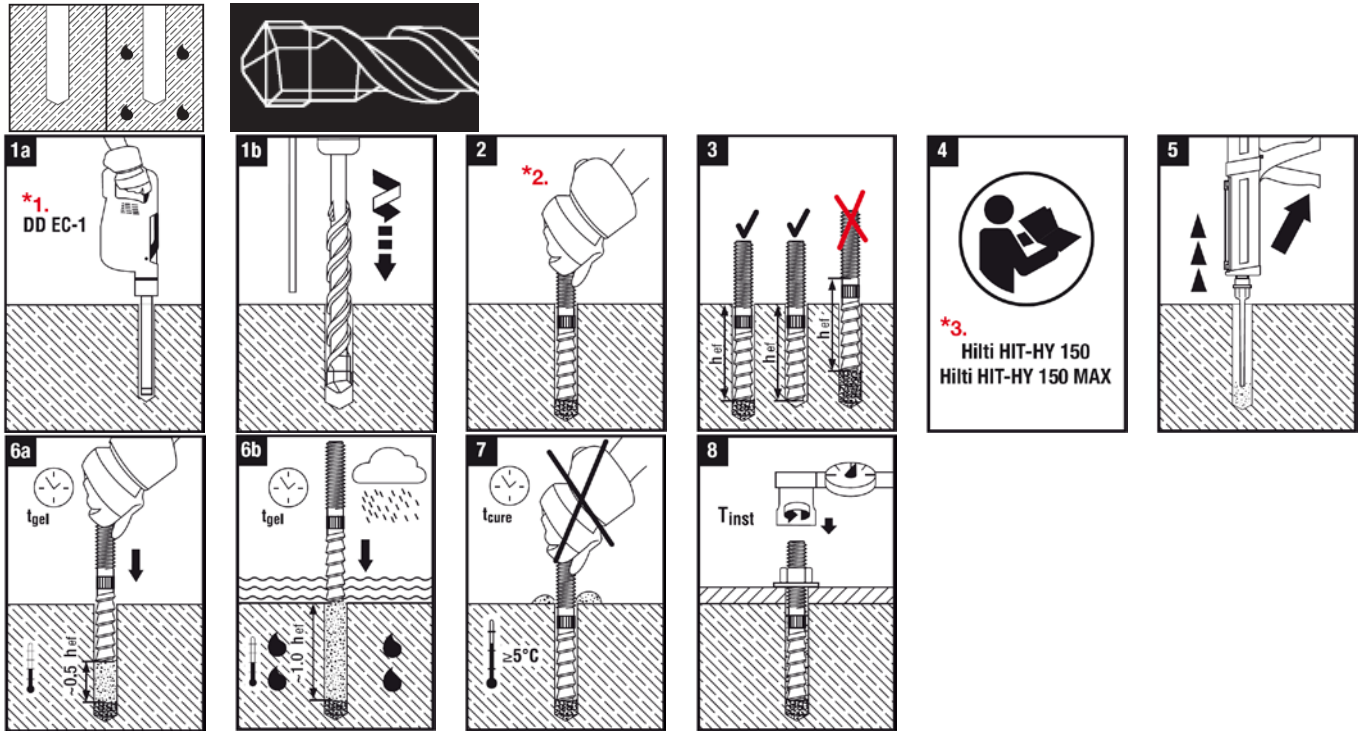
Setting

installation equipment

Anchor size	M8	M10	M12	M16	M20
Rotary hammer	TE 2 – TE 16				TE 40 - TE 70

Setting instruction

Dry, water-saturated concrete, under water, hammer drilling and diamond coring



1. Diamond coring is permissible only when the Hilti DD EC-1 diamond core drilling machine and the corresponding DD-C core bit are used.
2. Check the setting depth and compress the drilling dust. It is not necessary to clean the hole.
3. For use with Hilti HIT-HY 150 / Hilti HIT-HY 150 MAX. Read the instructions before use.

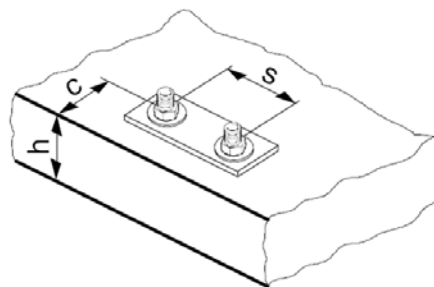
For detailed information on installation see instruction for use given with the package of the product.

Curing time for general conditions

Temperature of the base material	Curing time before anchor can be fully loaded t_{cure}
30 °C to 40 °C	30 min
20 °C to <30 °C	30 min
5 °C to <20 °C	60 min

Setting details

Anchor size			M8	M10	M12	M16	M20	
Nominal diameter of drill bit	d_0	[mm]	10	12	14	18	22	
Diameter of element	d	[mm]	8	10	12	16	20	
Effective anchorage depth	h_{ef}	[mm]	55	65	75	90	120	
Drill hole depth	h_0	[mm]	60	70	80	95	125	
Minimum base material thickness	$h_{min}^{a)}$	[mm]	110	130	150	180	240	
Diameter of clearance hole in the fixture	d_f	[mm]	9	12	14	18	22	
Non cracked concrete								
Minimum spacing	s_{min}	[mm]	40	50	55	70	80	
for	c	[mm]	50	70	75	80	90	
Minimum edge distance	c_{min}	[mm]	40	50	55	70	80	
for	s	[mm]	70	80	85	85	90	
Cracked concrete								
Minimum spacing	s_{min}	[mm]	40	60	70	80	100	
for	c	[mm]	65	85	100	100	120	
Minimum edge distance	c_{min}	[mm]	50	60	70	80	100	
for	s	[mm]	80	120	130	140	150	
Critical spacing for splitting failure	$s_{cr,sp}$	[mm]	$2 c_{cr,sp}$					
Critical edge distance for splitting failure	$c_{cr,sp}$	[mm]	$2 h_{ef}$					
Critical spacing for concrete cone failure	$s_{cr,N}$		$2 c_{cr,N}$					
Critical edge distance for concrete cone failure	$c_{cr,N}$		$1,5 h_{ef}$					
Torque moment	T_{inst}	[Nm]	12	23	40	70	130	



For spacing (edge distance) smaller than critical spacing (critical edge distance) the design loads have to be reduced.

a) h : base material thickness ($h \geq h_{min}$)

Simplified design method

Simplified version of the design method according ETAG 001, Annex C. Design resistance according data given in ETA-04/0084, issue 2009-12-09.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing
- Valid for a group of two anchors. (The method may also be applied for anchor groups with more than two anchors or more than one edge distance. The influencing factors must then be considered for each edge distance and spacing. The calculated design loads are then on the save side: They will be lower than the exact values according ETAG 001, Annex C. To avoid this, it is recommended to use the anchor design software PROFIS anchor)

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

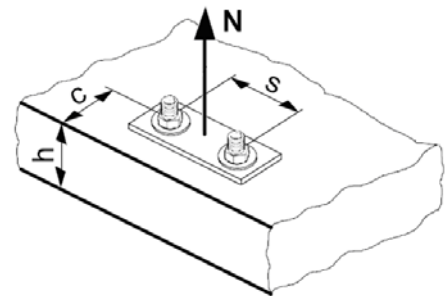
The values are valid for one anchor.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

Tension loading

The design tensile resistance is the lower value of

- Steel resistance: $N_{Rd,s}$
- Combined pull-out and concrete cone resistance: $N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{h,p}$
- Concrete cone resistance: $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$
- Concrete splitting resistance (only non-cracked concrete): $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,sp} \cdot f_{re,N}$



Basic design tensile resistance

Design steel resistance $N_{Rd,s}$

Anchor size	M8	M10	M12	M16	M20
$N_{Rd,s}$ HIT-TZ [kN]	14,7	23,3	34,0	62,7	98,0

Design combined pull-out and concrete cone resistance $N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{h,p}$

Anchor size	M8	M10	M12	M16	M20
Embedment depth h_{ef} [mm]	55	65	75	90	120
Non-cracked concrete					
$N_{Rd,p}^0$ Temperature range I [kN]	10,7	13,3	16,7	28,7	44,3
Cracked concrete					
$N_{Rd,p}^0$ Temperature range I [kN]	6,0	10,7	13,3	20,0	26,7

Design concrete cone resistance $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$

Design splitting resistance $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{h,N} \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{re,N}$

Anchor size	M8	M10	M12	M16	M20
$N_{Rd,c}^0$ Non cracked concrete [kN]	13,7	17,6	21,9	28,7	44,3
$N_{Rd,c}^0$ Cracked concrete [kN]	9,8	12,6	15,6	20,5	31,5

Influencing factors

Influence of concrete strength on combined pull-out and concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_{B,p} = (f_{ck,cube}/25N/mm^2)^{0.1}$ ^{a)}	1	1,02	1,04	1,06	1,07	1,08	1,09

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of embedment depth on combined pull-out and concrete cone resistance

$f_{h,p} = h_{ef}/h_{ef,typ}$

Influence of concrete strength on concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{1/2}$ ^{a)}	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of edge distance ^{a)}

$c/c_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$c/c_{cr,sp}$										
$f_{1,N} = 0,7 + 0,3 \cdot c/c_{cr,N}$	0,73	0,76	0,79	0,82	0,85	0,88	0,91	0,94	0,97	1
$f_{1,sp} = 0,7 + 0,3 \cdot c/c_{cr,sp}$										
$f_{2,N} = 0,5 \cdot (1 + c/c_{cr,N})$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{2,sp} = 0,5 \cdot (1 + c/c_{cr,sp})$										

a) The edge distance shall not be smaller than the minimum edge distance c_{min} given in the table with the setting details. These influencing factors must be considered for every edge distance smaller than the critical edge distance.

Influence of anchor spacing ^{a)}

$s/s_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$s/s_{cr,sp}$										
$f_{3,N} = 0,5 \cdot (1 + s/s_{cr,N})$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{3,sp} = 0,5 \cdot (1 + s/s_{cr,sp})$										

a) The anchor spacing shall not be smaller than the minimum anchor spacing s_{min} given in the table with the setting details. This influencing factor must be considered for every anchor spacing.

Influence of embedment depth on concrete cone resistance

$f_{h,N} = (h_{ef}/h_{ef,typ})^{1,5}$

Influence of reinforcement

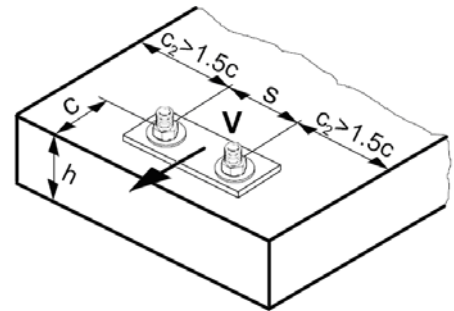
h_{ef} [mm]	80	90	≥ 100
$f_{re,N} = 0,5 + h_{ef}/200mm \leq 1$	0,9 ^{a)}	0,95 ^{a)}	1

a) This factor applies only for dense reinforcement. If in the area of anchorage there is reinforcement with a spacing ≥ 150 mm (any diameter) or with a diameter ≤ 10 mm and a spacing ≥ 100 mm, then a factor $f_{re} = 1$ may be applied.

Shear loading

The design shear resistance is the lower value of

- Steel resistance: $V_{Rd,s}$
- Concrete pryout resistance: $V_{Rd,cp} = k \cdot \text{lower value of } N_{Rd,p} \text{ and } N_{Rd,c}$
- Concrete edge resistance: $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_{\beta} \cdot f_h \cdot f_4$



Basic design shear resistance

Design steel resistance $V_{Rd,s}$

Anchor size	M8	M10	M12	M16	M20
$V_{Rd,s}$ HIT-(R)TZ [kN]	8,8	13,6	20,0	37,6	59,2

Design concrete pryout resistance $V_{Rd,cp} = \text{lower value}^a)$ of $k \cdot N_{Rd,p}$ and $k \cdot N_{Rd,c}$

$$k = 1 \text{ for } h_{ef} < 60 \text{ mm}$$

$$k = 2 \text{ for } h_{ef} \geq 60 \text{ mm}$$

- a) $N_{Rd,p}$: Design combined pull-out and concrete cone resistance
 $N_{Rd,c}$: Design concrete cone resistance

Design concrete edge resistance $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_{\beta} \cdot f_h \cdot f_4$

Anchor size	Non-cracked concrete					Cracked concrete				
	M8	M10	M12	M16	M20	M8	M10	M12	M16	M20
$V_{Rd,c}^0$ [kN]	3,1	4,5	6,1	8,5	13,4	1,6	2,4	3,1	5,0	6,9

- a) For anchor groups only the anchors close to the edge must be considered.

Influencing factors

Influence of concrete strength

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{1/2}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

- a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of angle between load applied and the direction perpendicular to the free edge

Angle β	0°	10°	20°	30°	40°	50°	60°	70°	80°	≥ 90°
$f_{\beta} = \frac{1}{\sqrt{(\cos \alpha_V)^2 + \left(\frac{\sin \alpha_V}{2,5}\right)^2}}$	1	1,01	1,05	1,13	1,24	1,40	1,64	1,97	2,32	2,50

Influence of base material thickness

h/c	0,15	0,3	0,45	0,6	0,75	0,9	1,05	1,2	1,35	≥ 1,5
$f_h = \{h/(1,5 \cdot c)\}^{2/3} \leq 1$	0,22	0,34	0,45	0,54	0,63	0,71	0,79	0,86	0,93	1,00

Influence of anchor spacing and edge distance ^{a)} for concrete edge resistance: f_4

$$f_4 = (c/h_{ef})^{1,5} \cdot (1 + s / [3 \cdot c]) \cdot 0,5$$

c/h _{ef}	Single anchor	Group of two anchors s/h _{ef}														
		0,75	1,50	2,25	3,00	3,75	4,50	5,25	6,00	6,75	7,50	8,25	9,00	9,75	10,50	11,25
0,50	0,35	0,27	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35
0,75	0,65	0,43	0,54	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65
1,00	1,00	0,63	0,75	0,88	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00
1,25	1,40	0,84	0,98	1,12	1,26	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40
1,50	1,84	1,07	1,22	1,38	1,53	1,68	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84
1,75	2,32	1,32	1,49	1,65	1,82	1,98	2,15	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32
2,00	2,83	1,59	1,77	1,94	2,12	2,30	2,47	2,65	2,83	2,83	2,83	2,83	2,83	2,83	2,83	2,83
2,25	3,38	1,88	2,06	2,25	2,44	2,63	2,81	3,00	3,19	3,38	3,38	3,38	3,38	3,38	3,38	3,38
2,50	3,95	2,17	2,37	2,57	2,77	2,96	3,16	3,36	3,56	3,76	3,95	3,95	3,95	3,95	3,95	3,95
2,75	4,56	2,49	2,69	2,90	3,11	3,32	3,52	3,73	3,94	4,15	4,35	4,56	4,56	4,56	4,56	4,56
3,00	5,20	2,81	3,03	3,25	3,46	3,68	3,90	4,11	4,33	4,55	4,76	4,98	5,20	5,20	5,20	5,20
3,25	5,86	3,15	3,38	3,61	3,83	4,06	4,28	4,51	4,73	4,96	5,18	5,41	5,63	5,86	5,86	5,86
3,50	6,55	3,51	3,74	3,98	4,21	4,44	4,68	4,91	5,14	5,38	5,61	5,85	6,08	6,31	6,55	6,55
3,75	7,26	3,87	4,12	4,36	4,60	4,84	5,08	5,33	5,57	5,81	6,05	6,29	6,54	6,78	7,02	7,26
4,00	8,00	4,25	4,50	4,75	5,00	5,25	5,50	5,75	6,00	6,25	6,50	6,75	7,00	7,25	7,50	7,75
4,25	8,76	4,64	4,90	5,15	5,41	5,67	5,93	6,18	6,44	6,70	6,96	7,22	7,47	7,73	7,99	8,25
4,50	9,55	5,04	5,30	5,57	5,83	6,10	6,36	6,63	6,89	7,16	7,42	7,69	7,95	8,22	8,49	8,75
4,75	10,35	5,45	5,72	5,99	6,27	6,54	6,81	7,08	7,36	7,63	7,90	8,17	8,45	8,72	8,99	9,26
5,00	11,18	5,87	6,15	6,43	6,71	6,99	7,27	7,55	7,83	8,11	8,39	8,66	8,94	9,22	9,50	9,78
5,25	12,03	6,30	6,59	6,87	7,16	7,45	7,73	8,02	8,31	8,59	8,88	9,17	9,45	9,74	10,02	10,31
5,50	12,90	6,74	7,04	7,33	7,62	7,92	8,21	8,50	8,79	9,09	9,38	9,67	9,97	10,26	10,55	10,85

a) The anchor spacing and the edge distance shall not be smaller than the minimum anchor spacing s_{min} and the minimum edge distance c_{min} .

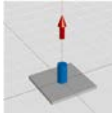
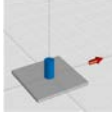
Combined tension and shear loading

For combined tension and shear loading see section "Anchor Design".

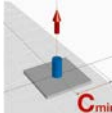
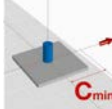
Precalculated values

Recommended loads can be calculated by dividing the design resistance by an overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$

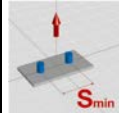
Anchor size		M8	M10	M12	M16	M20
Embedment depth $h_{ef} =$ [mm]		55	65	75	90	120
Base material thickness $h_{min} =$ [mm]		110	130	150	180	240
	Tensile N_{Rd}: single anchor, no edge effects					
	Non-cracked concrete					
	HIT-(R)TZ [kN]	10,7	13,3	16,7	28,7	44,3
	Cracked concrete					
	HIT-(R)TZ [kN]	6,0	10,7	13,3	20,0	26,7
		Shear V_{Rd}: single anchor, no edge effects, without lever arm				
Non-cracked concrete						
HIT-(R)TZ [kN]		8,8	13,6	20,0	37,6	59,2
Cracked concrete						
HIT-(R)TZ [kN]		6,0	13,6	20,0	37,6	53,3

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$





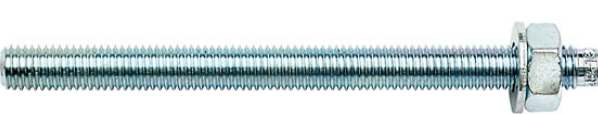
Anchor size		M8	M10	M12	M16	M20
Embedment depth $h_{ef} =$ [mm]		55	65	75	90	120
Base material thickness $h_{min} =$ [mm]		110	130	150	180	240
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)					
	Non-cracked concrete					
	c_{min} [mm]	50	60	70	80	100
	HIT-(R)TZ [kN]	6,5	8,2	10,3	17,3	25,9
	Cracked concrete					
	c_{min} [mm]	40	50	55	70	80
HIT-(R)TZ [kN]	3,3	6,0	7,4	11,3	14,2	
	Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm					
	Non-cracked concrete					
	c_{min} [mm]	50	60	70	80	100
	HIT-(R)TZ [kN]	3,1	4,5	6,1	8,5	13,4
	Cracked concrete					
	c_{min} [mm]	40	50	55	70	80
HIT-(R)TZ [kN]	1,6	2,4	3,1	5,0	6,9	

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$
(load values are valid for single anchor)

Anchor size		M8	M10	M12	M16	M20
Embedment depth $h_{ef} =$ [mm]		55	65	75	90	120
Base material thickness $h_{min} =$ [mm]		110	130	150	180	240
Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)						
Non-cracked concrete						
s_{min}	[mm]	40	60	70	80	100
HIT-(R)TZ	[kN]	6,3	8,2	10,3	17,6	26,7
Cracked concrete						
s_{min}	[mm]	40	50	55	70	80
HIT-(R)TZ	[kN]	3,5	6,4	7,9	11,9	15,6
Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$) , without lever arm						
Non-cracked concrete						
s_{min}	[mm]	40	60	70	80	100
HIT-(R)TZ	[kN]	6,6	13,6	20,0	37,3	56,5
Cracked concrete						
s_{min}	[mm]	40	50	55	70	80
HIT-(R)TZ	[kN]	3,7	13,4	16,6	25,2	32,6



Hilti HIT-HY 150 MAX with HIT-V / HAS

Injection mortar system		Benefits
	Hilti HIT-HY 150 MAX 330 ml foil pack (also available as 500 ml and 1400 ml foil pack)	<ul style="list-style-type: none"> - suitable for non-cracked concrete and cracked concrete C 20/25 to C 50/60 - suitable for dry and water saturated concrete - high loading capacity - rapid curing - small edge distance and anchor spacing possible - large diameter applications - high corrosion resistant - in service temperature range up to 120°C short term/ 72°C long term - manual cleaning for anchor size M8 to M16 and embedment depth $h_{ef} \leq 10d$ for non-cracked concrete only - embedment depth range: from 60 ... 160 mm for M8 to 120 ... 600 mm for M30
	Static mixer	
	HAS rods HAS-R rods HAS-HCR rods	
	HAS-E rods HAS-E-R rods	
	HIT-V rods HIT-V-R rods HIT-V-HCR rods	



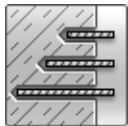
Concrete



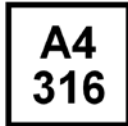
Tensile zone



Small edge distance and spacing



Variable embedment depth



Corrosion resistance



High corrosion resistance



European Technical Approval



CE conformity



PROFIS Anchor design software

Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval ^{a)}	DIBt, Berlin	ETA-08/0352 / 2010-04-01
Fire test report	MFPA, Leipzig	GS 3.2/09-121 Ä / 2011-08-19

a) All data given in this section according ETA-08/0352 issue 2010-04-01.

Basic loading data (for a single anchor)

All data in this section applies to

For details see Simplified design method

- Correct setting (See setting instruction)
- No edge distance and spacing influence
- Steel failure
- Base material thickness, as specified in the table
- One typical embedment depth, as specified in the table
- One anchor material, as specified in the tables
- Concrete C 20/25, $f_{ck,cube} = 25 \text{ N/mm}^2$
- Temperate range I
(min. base material temperature -40°C , max. long term/short term base material temperature: $+24^\circ\text{C}/40^\circ\text{C}$)
- Installation temperature range -10°C to $+40^\circ\text{C}$

Embedment depth ^{a)} and base material thickness for the basic loading data. Mean ultimate resistance, characteristic resistance, design resistance, recommended loads.

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Typical embedment depth h_{ef} [mm]	80	90	110	125	170	210	240	270
Base material thickness h [mm]	110	120	140	165	220	270	300	340

a) The allowed range of embedment depth is shown in the setting details. The corresponding load values can be calculated according to the simplified design method.

Mean ultimate resistance: concrete C 20/25 , anchor HIT-V 5.8

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete								
Tensile $N_{R_{u,m}}$ HIT-V 5.8 [kN]	18,9	30,5	44,1	83,0	129,2	185,9	241,5	288,4
Shear $V_{R_{u,m}}$ HIT-V 5.8 [kN]	9,5	15,8	22,1	41,0	64,1	92,4	120,8	147,0
Cracked concrete								
Tensile $N_{R_{u,m}}$ HIT-V 5.8 [kN]	-	20,7	30,4	50,3	85,5	126,7	-	-
Shear $V_{R_{u,m}}$ HIT-V 5.8 [kN]	-	15,8	22,1	41,0	64,1	92,4	-	-

Characteristic resistance: concrete C 20/25 , anchor HIT-V 5.8

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete								
Tensile N_{R_k} HIT-V 5.8 [kN]	18,0	29,0	42,0	70,6	111,9	153,7	187,8	216,3
Shear V_{R_k} HIT-V 5.8 [kN]	9,0	15,0	21,0	39,0	61,0	88,0	115,0	140,0
Cracked concrete								
Tensile N_{R_k} HIT-V 5.8 [kN]	-	15,6	22,8	37,7	64,1	95,0	-	-
Shear V_{R_k} HIT-V 5.8 [kN]	-	15,0	21,0	39,0	61,0	88,0	-	-

Design resistance: concrete C 20/25 , anchor HIT-V 5.8

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete								
Tensile N_{R_d} HIT-V 5.8 [kN]	12,0	19,3	28,0	47,1	74,6	102,5	125,2	120,2
Shear V_{R_d} HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
Cracked concrete								
Tensile N_{R_d} HIT-V 5.8 [kN]	-	10,4	15,2	25,1	42,7	63,3	-	-
Shear V_{R_d} HIT-V 5.8 [kN]	-	12,0	16,8	31,2	48,8	70,4	-	-

Recommended loads ^{a)}: concrete C 20/25 , anchor HIT-V 5.8

Anchor size			M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete										
Tensile N _{rec}	HIT-V 5.8	[kN]	8,6	13,8	20,0	33,6	53,3	73,2	89,4	85,8
Shear V _{rec}	HIT-V 5.8	[kN]	5,1	8,6	12,0	22,3	34,9	50,3	65,7	80,0
Cracked concrete										
Tensile N _{rec}	HIT-V 5.8	[kN]	-	7,4	10,9	18,0	30,5	45,2	-	-
Shear V _{rec}	HIT-V 5.8	[kN]	-	8,6	12,0	22,3	34,9	50,3	-	-

a) With overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Service temperature range

Hilti HIT-HY 150 MAX injection mortar may be applied in the temperature ranges given below. An elevated base material temperature may lead to a reduction of the design bond resistance.

Temperature range	Base material temperature	Maximum long term base material temperature	Maximum short term base material temperature
Temperature range I	-40 °C to +40 °C	+24 °C	+40 °C
Temperature range II	-40 °C to +80 °C	+50 °C	+80 °C
Temperature range III	-40 °C to +120 °C	+72 °C	+120 °C

Max short term base material temperature

Short-term elevated base material temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Max long term base material temperature

Long-term elevated base material temperatures are roughly constant over significant periods of time.

Materials

Mechanical properties of HIT-V / HAS

Anchor size			M8	M10	M12	M16	M20	M24	M27	M30
Nominal tensile strength f _{uk}	HIT-V/HAS 5.8	[N/mm ²]	500	500	500	500	500	500	500	500
	HIT-V/HAS 8.8	[N/mm ²]	800	800	800	800	800	800	800	800
	HIT-V/HAS -R	[N/mm ²]	700	700	700	700	700	700	500	500
	HIT-V/HAS -HCR	[N/mm ²]	800	800	800	800	800	700	700	700
Yield strength f _{yk}	HIT-V/HAS 5.8	[N/mm ²]	400	400	400	400	400	400	400	400
	HIT-V/HAS 8.8	[N/mm ²]	640	640	640	640	640	640	640	640
	HIT-V/HAS -R	[N/mm ²]	450	450	450	450	450	450	210	210
	HIT-V/HAS -HCR	[N/mm ²]	600	600	600	600	600	400	400	400
Stressed cross-section A _s	HAS	[mm ²]	32,8	52,3	76,2	144	225	324	427	519
	HIT-V	[mm ²]	36,6	58,0	84,3	157	245	353	459	561
Moment of resistance W	HAS	[mm ³]	27,0	54,1	93,8	244	474	809	1274	1706
	HIT-V	[mm ³]	31,2	62,3	109	277	541	935	1387	1874

Material quality

Part	Material
Threaded rod HIT-V(F), HAS 5.8: M8 – M24	Strength class 5.8, A ₅ > 8% ductile steel galvanized ≥ 5 µm, (F) hot dipped galvanized ≥ 45 µm,
Threaded rod HIT-V(F), HAS 8.8 M27 – M30	Strength class 8.8, A ₅ > 8% ductile steel galvanized ≥ 5 µm, (F) hot dipped galvanized ≥ 45 µm,
Threaded rod HIT-V-R, HAS-R	Stainless steel grade A4, A ₅ > 8% ductile strength class 70 for ≤ M24 and class 50 for M27 to M30, 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362
Threaded rod HIT-V-HCR, HAS-HCR	High corrosion resistant steel, 1.4529; 1.4565 strength ≤ M20: R _m = 800 N/mm ² , R _{p0.2} = 640 N/mm ² , A ₅ > 8% ductile M24 to M30: R _m = 700 N/mm ² , R _{p0.2} = 400 N/mm ² , A ₅ > 8% ductile
Washer ISO 7089	Steel galvanized, hot dipped galvanized,
	Stainless steel, 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362
	High corrosion resistant steel, 1.4529; 1.4565
Nut EN ISO 4032	Strength class 8, steel galvanized ≥ 5 µm, hot dipped galvanized ≥ 45 µm,
	Strength class 70, stainless steel grade A4, 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362
	Strength class 70, high corrosion resistant steel, 1.4529; 1.4565

Anchor dimensions

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Anchor rod HAS, HAS-R, HAS-HCR HAS-E, HAS-E-R	M8x80	M10x90	M12x110	M16x125	M20x170	M24x210	M27x240	M30x270
Embedment depth h _{ef} [mm]	80	90	110	125	170	210	240	270
Anchor rod HIT-V, HIT-V-R, HIT-V-HCR	Anchor rods HIT-V (-R / -HCR) are available in variable length							

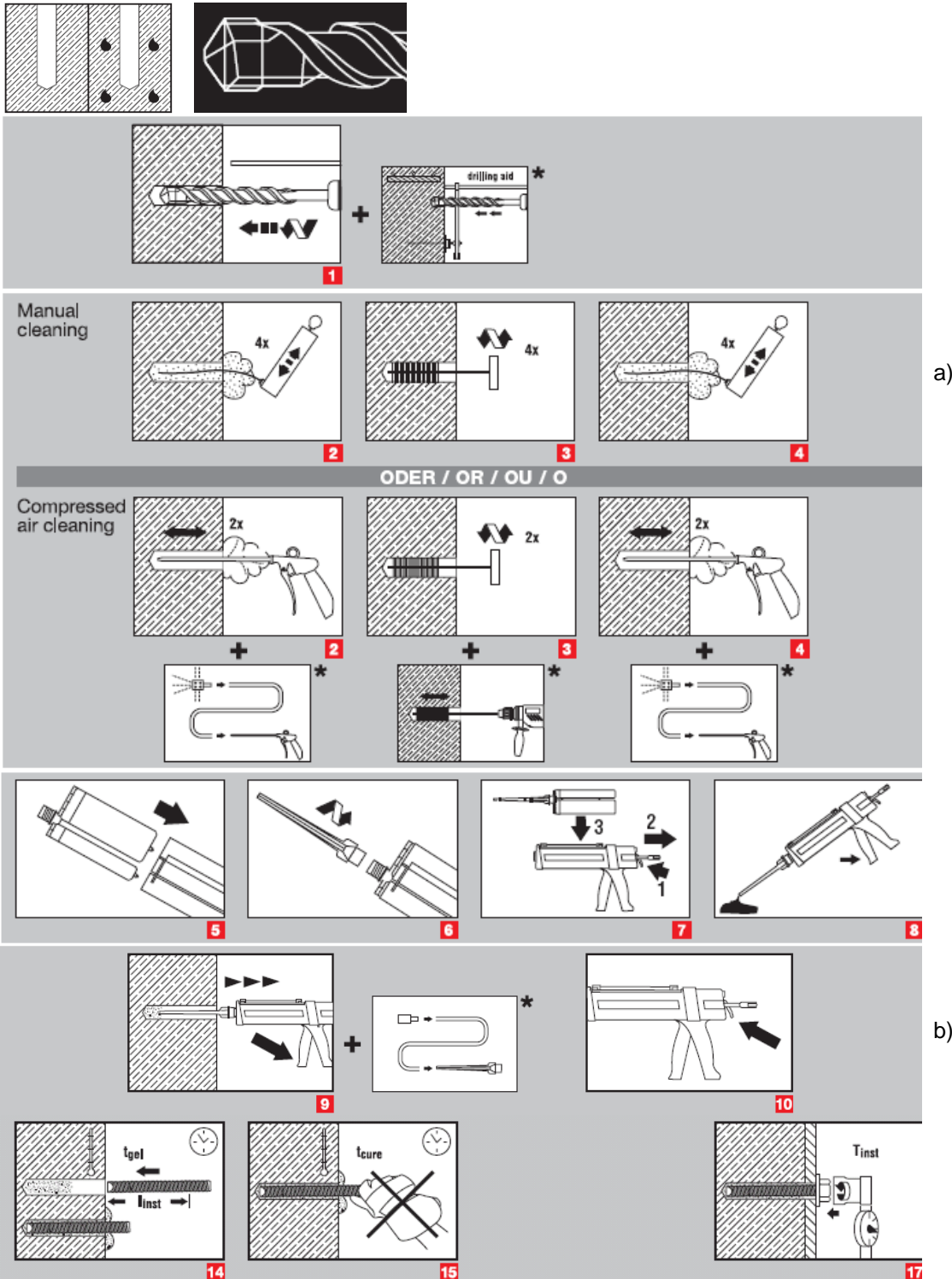
Setting

installation equipment

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Rotary hammer	TE 2 – TE 16				TE 40 – TE 70			
Other tools	compressed air gun or blow out pump, set of cleaning brushes, dispenser							

Setting instruction

Dry and water-saturated concrete, hammer drilling



a) Note: Manual cleaning for non-cracked concrete, element sizes $d \leq 16\text{mm}$ and embedment depth $h_{ef} \leq 10 d$ only!

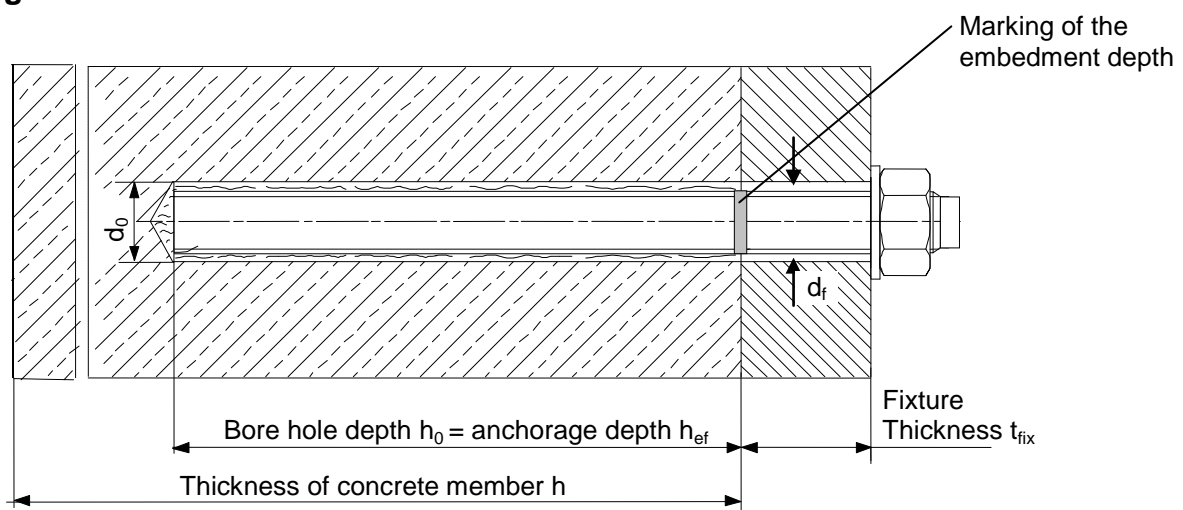
b) Note: Extension and piston plug needed for overhead installation and/or embedment depth $> 250\text{mm}$!

For detailed information on installation see instruction for use given with the package of the product.

Working time, Curing time

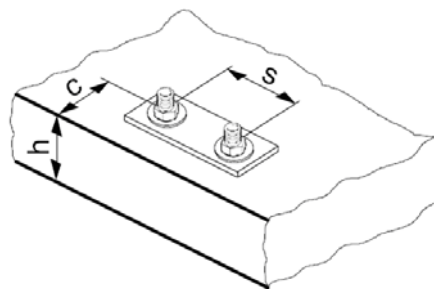
Temperature of the base material T_{BM}	Working time t_{gel}	Curing time t_{cure}
$-10\text{ °C} \leq T_{BM} < -5\text{ °C}$	180 min	12 h
$-5\text{ °C} \leq T_{BM} < 0\text{ °C}$	40 min	4 h
$0\text{ °C} \leq T_{BM} < 5\text{ °C}$	20 min	2 h
$5\text{ °C} \leq T_{BM} < 20\text{ °C}$	8 min	1 h
$20\text{ °C} \leq T_{BM} < 30\text{ °C}$	5 min	30 min
$30\text{ °C} \leq T_{BM} \leq 40\text{ °C}$	2 min	30 min

Setting details



Setting details

Anchor size			M8	M10	M12	M16	M20	M24	M27	M30
Nominal diameter of drill bit	d_0	[mm]	10	12	14	18	24	28	30	35
Effective embedment and drill hole depth range ^{a)} for HIT-V	$h_{ef,min}$	[mm]	60	60	70	80	90	100	110	120
	$h_{ef,max}$	[mm]	160	200	240	320	400	480	540	600
Effective anchorage and drill hole depth for HAS	h_{ef}	[mm]	80	90	110	125	170	210	240	270
Minimum base material thickness	h_{min}	[mm]	$h_{ef} + 30 \text{ mm}$ $\geq 100 \text{ mm}$			$h_{ef} + 2 d_0$				
Diameter of clearance hole in the fixture	d_f	[mm]	9	12	14	18	22	26	30	33
Torque moment	T_{max} ^{b)}	[Nm]	10	20	40	80	150	200	270	300
Minimum spacing	s_{min}	[mm]	40	50	60	80	100	120	135	150
Minimum edge distance	c_{min}	[mm]	40	50	60	80	100	120	135	150
Critical spacing for splitting failure	$s_{cr,sp}$	[mm]	$2 c_{cr,sp}$							
Critical edge distance for splitting failure ^{c)}	$c_{cr,sp}$	[mm]	$1,0 \cdot h_{ef}$ for $h / h_{ef} \geq 2,0$							
			$4,6 h_{ef} - 1,8 h$ for $2,0 > h / h_{ef} > 1,3$							
			$2,26 h_{ef}$ for $h / h_{ef} \leq 1,3$							
Critical spacing for concrete cone failure	$s_{cr,N}$	[mm]	$2 c_{cr,N}$							
Critical edge distance for concrete cone failure ^{d)}	$c_{cr,N}$	[mm]	$1,5 h_{ef}$							



For spacing (or edge distance) smaller than critical spacing (or critical edge distance) the design loads have to be reduced.

- a) Embedment depth range: $h_{ef,min} \leq h_{ef} \leq h_{ef,max}$
- b) Maximum recommended torque moment to avoid splitting failure during installation with minimum spacing and/or edge distance.
- c) h : base material thickness ($h \geq h_{min}$), h_{ef} : embedment depth
- d) The critical edge distance for concrete cone failure depends on the embedment depth h_{ef} and the design bond resistance. The simplified formula given in this table is on the save side.

Simplified design method

Simplified version of the design method according ETAG 001, TR 029. Design resistance according data given in ETA-08/0352, issue 2010-04-01.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing
- Valid for a group of two anchors. (The method may also be applied for anchor groups with more than two anchors or more than one edge distance. The influencing factors must then be considered for each edge distance and spacing. The calculated design loads are then on the safe side: They will be lower than the exact values according ETAG 001, TR 029. To avoid this, it is recommended to use the anchor design software PROFIS anchor)

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

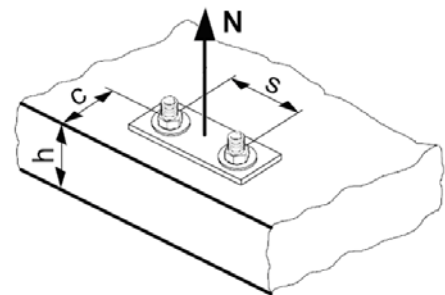
The values are valid for one anchor.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

TENSION loading

The design tensile resistance is the lower value of

- Steel resistance: $N_{Rd,s}$
- Combined pull-out and concrete cone resistance: $N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$
- Concrete cone resistance: $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$
- Concrete splitting resistance (only non-cracked concrete): $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$



Basic design tensile resistance

Design steel resistance $N_{Rd,s}$

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
$N_{Rd,s}$	HAS 5.8 [kN]	11,3	17,3	25,3	48,0	74,7	106,7	-	-
	HIT-V 5.8 [kN]	12,0	19,3	28,0	52,7	82,0	118,0	153,3	187,3
	HAS 8.8 [kN]	-	-	-	-	-	-	231,3	281,3
	HIT-V 8.8 [kN]	19,3	30,7	44,7	84,0	130,7	188,0	244,7	299,3
	HAS (-E)-R [kN]	12,3	19,8	28,3	54,0	84,0	119,8	75,9	92,0
	HIT-V-R [kN]	13,9	21,9	31,6	58,8	92,0	132,1	80,4	98,3
	HAS (-E)-HCR [kN]	18,0	28,0	40,7	76,7	120,0	106,7	144,8	175,7
	HIT-V-HCR [kN]	19,3	30,7	44,7	84,0	130,7	117,6	152,9	187,1

Design combined pull-out and concrete cone resistance

$$N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$$

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Typical embedment depth $h_{ef} = h_{ef,typ}$ [mm]	80	90	110	125	170	210	240	270
Non-cracked concrete								
$N_{Rd,p}^0$ Temperature range I [kN]	15,6	22,0	32,3	54,5	85,5	116,1	135,7	120,2
$N_{Rd,p}^0$ Temperature range II [kN]	13,4	18,8	27,6	50,3	78,3	105,6	122,1	99,0
$N_{Rd,p}^0$ Temperature range III [kN]	8,9	12,6	18,4	29,3	46,3	63,3	74,6	63,6
Cracked concrete								
$N_{Rd,p}^0$ Temperature range I [kN]	-	10,4	15,2	25,1	42,7	63,3	-	-
$N_{Rd,p}^0$ Temperature range II [kN]	-	8,5	13,8	23,0	42,7	63,3	-	-
$N_{Rd,p}^0$ Temperature range III [kN]	-	5,7	8,3	14,7	24,9	42,2	-	-

Design concrete cone resistance $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$

Design splitting resistance ^{a)} $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
$N_{Rd,c}^0$ Non-cracked concrete [kN]	20,1	24,0	32,4	47,1	74,6	102,5	125,2	124,5
$N_{Rd,c}^0$ Cracked concrete [kN]	-	20,5	27,7	33,5	53,2	73,0	-	-

a) Splitting resistance must only be considered for non-cracked concrete.

Influencing factors

Influence of concrete strength on combined pull-out and concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_{B,p} = (f_{ck,cube}/25N/mm^2)^{0,10}$ ^{a)}	1,00	1,02	1,04	1,06	1,07	1,08	1,09

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of embedment depth on combined pull-out and concrete cone resistance

$$f_{h,p} = h_{ef}/h_{ef,typ}$$

Influence of concrete strength on concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{0,5}$ ^{a)}	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of edge distance ^{a)}

$c/c_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$c/c_{cr,sp}$										
$f_{1,N} = 0,7 + 0,3 \cdot c/c_{cr,N} \leq 1$	0,73	0,76	0,79	0,82	0,85	0,88	0,91	0,94	0,97	1
$f_{1,sp} = 0,7 + 0,3 \cdot c/c_{cr,sp} \leq 1$										
$f_{2,N} = 0,5 \cdot (1 + c/c_{cr,N}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{2,sp} = 0,5 \cdot (1 + c/c_{cr,sp}) \leq 1$										

a) The edge distance shall not be smaller than the minimum edge distance c_{min} . These influencing factors must be considered for every edge distance smaller than the critical edge distance.

Influence of anchor spacing ^{a)}

$s/s_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$f_{3,N} = 0,5 \cdot (1 + s/s_{cr,N}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{3,sp} = 0,5 \cdot (1 + s/s_{cr,sp}) \leq 1$										

a) The anchor spacing shall not be smaller than the minimum anchor spacing s_{min} . This influencing factor must be considered for every anchor spacing.

Influence of embedment depth on concrete cone resistance

$$f_{h,N} = (h_{ef}/h_{ef,typ})^{1,5}$$

Influence of reinforcement

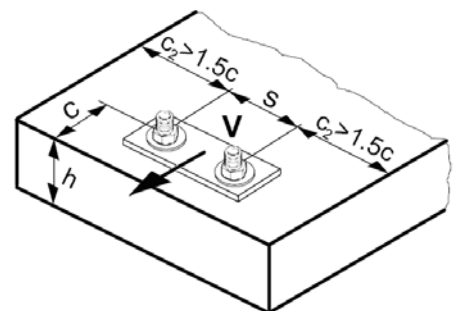
h_{ef} [mm]	40	50	60	70	80	90	≥ 100
$f_{re,N} = 0,5 + h_{ef}/200\text{mm} \leq 1$	0,7 ^{a)}	0,75 ^{a)}	0,8 ^{a)}	0,85 ^{a)}	0,9 ^{a)}	0,95 ^{a)}	1

a) This factor applies only for dense reinforcement. If in the area of anchorage there is reinforcement with a spacing ≥ 150 mm (any diameter) or with a diameter ≤ 10 mm and a spacing ≥ 100 mm, then a factor $f_{re,N} = 1$ may be applied.

SHEAR loading

The design shear resistance is the lower value of

- Steel resistance: $V_{Rd,s}$
- Concrete pryout resistance: $V_{Rd,cp} = k \cdot \text{lower value of } N_{Rd,p} \text{ and } N_{Rd,c}$
- Concrete edge resistance: $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$



Basic design shear resistance

Design steel resistance $V_{Rd,s}$

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
$V_{Rd,s}$	HAS 5.8 [kN]	6,8	10,4	15,2	28,8	44,8	64,0	-	-
	HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
	HAS 8.8 [kN]	-	-	-	-	-	-	139,2	168,8
	HIT-V 8.8 [kN]	12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2
	HAS (-E)-R [kN]	7,7	12,2	17,3	32,7	50,6	71,8	45,8	55,5
	HIT-V-R [kN]	8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
	HAS (-E)-HCR [kN]	10,4	16,8	24,8	46,4	72,0	64,0	86,9	105,7
	HIT-V-HCR [kN]	12,0	18,4	27,2	50,4	78,4	70,9	92,0	112,0

Design concrete pryout resistance $V_{Rd,cp} = \text{lower value}^a)$ of $k \cdot N_{Rd,p}$ and $k \cdot N_{Rd,c}$

$$k = 2 \text{ for } h_{ef} \geq 60 \text{ mm}$$

- a) $N_{Rd,p}$: Design combined pull-out and concrete cone resistance
 $N_{Rd,c}$: Design concrete cone resistance

$$\text{Design concrete edge resistance } V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_{\beta} \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$$

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete									
$V_{Rd,c}^0$	[kN]	5,9	8,6	11,6	18,7	27,0	36,6	44,5	53,0
Cracked concrete									
$V_{Rd,c}^0$	[kN]	-	6,1	8,2	13,2	19,2	25,9	31,5	-

Influencing factors

Influence of concrete strength

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25\text{N/mm}^2)^{1/2}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of angle between load applied and the direction perpendicular to the free edge

Angle β	0°	10°	20°	30°	40°	50°	60°	70°	80°	≥ 90°
$f_{\beta} = \frac{1}{\sqrt{(\cos \alpha_v)^2 + \left(\frac{\sin \alpha_v}{2,5}\right)^2}}$	1	1,01	1,05	1,13	1,24	1,40	1,64	1,97	2,32	2,50

Influence of base material thickness

h/c	0,15	0,3	0,45	0,6	0,75	0,9	1,05	1,2	1,35	≥ 1,5
$f_h = \{h/(1,5 \cdot c)\}^{1/2} \leq 1$	0,32	0,45	0,55	0,63	0,71	0,77	0,84	0,89	0,95	1,00

Influence of anchor spacing and edge distance ^{a)} for concrete edge resistance: f_4

$$f_4 = (c/h_{ef})^{1,5} \cdot (1 + s / [3 \cdot c]) \cdot 0,5$$

c/h _{ef}	Single anchor	Group of two anchors s/h _{ef}														
		0,75	1,50	2,25	3,00	3,75	4,50	5,25	6,00	6,75	7,50	8,25	9,00	9,75	10,50	11,25
0,50	0,35	0,27	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35
0,75	0,65	0,43	0,54	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65
1,00	1,00	0,63	0,75	0,88	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00
1,25	1,40	0,84	0,98	1,12	1,26	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40
1,50	1,84	1,07	1,22	1,38	1,53	1,68	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84
1,75	2,32	1,32	1,49	1,65	1,82	1,98	2,15	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32
2,00	2,83	1,59	1,77	1,94	2,12	2,30	2,47	2,65	2,83	2,83	2,83	2,83	2,83	2,83	2,83	2,83
2,25	3,38	1,88	2,06	2,25	2,44	2,63	2,81	3,00	3,19	3,38	3,38	3,38	3,38	3,38	3,38	3,38
2,50	3,95	2,17	2,37	2,57	2,77	2,96	3,16	3,36	3,56	3,76	3,95	3,95	3,95	3,95	3,95	3,95
2,75	4,56	2,49	2,69	2,90	3,11	3,32	3,52	3,73	3,94	4,15	4,35	4,56	4,56	4,56	4,56	4,56
3,00	5,20	2,81	3,03	3,25	3,46	3,68	3,90	4,11	4,33	4,55	4,76	4,98	5,20	5,20	5,20	5,20
3,25	5,86	3,15	3,38	3,61	3,83	4,06	4,28	4,51	4,73	4,96	5,18	5,41	5,63	5,86	5,86	5,86
3,50	6,55	3,51	3,74	3,98	4,21	4,44	4,68	4,91	5,14	5,38	5,61	5,85	6,08	6,31	6,55	6,55
3,75	7,26	3,87	4,12	4,36	4,60	4,84	5,08	5,33	5,57	5,81	6,05	6,29	6,54	6,78	7,02	7,26
4,00	8,00	4,25	4,50	4,75	5,00	5,25	5,50	5,75	6,00	6,25	6,50	6,75	7,00	7,25	7,50	7,75
4,25	8,76	4,64	4,90	5,15	5,41	5,67	5,93	6,18	6,44	6,70	6,96	7,22	7,47	7,73	7,99	8,25
4,50	9,55	5,04	5,30	5,57	5,83	6,10	6,36	6,63	6,89	7,16	7,42	7,69	7,95	8,22	8,49	8,75
4,75	10,35	5,45	5,72	5,99	6,27	6,54	6,81	7,08	7,36	7,63	7,90	8,17	8,45	8,72	8,99	9,26
5,00	11,18	5,87	6,15	6,43	6,71	6,99	7,27	7,55	7,83	8,11	8,39	8,66	8,94	9,22	9,50	9,78
5,25	12,03	6,30	6,59	6,87	7,16	7,45	7,73	8,02	8,31	8,59	8,88	9,17	9,45	9,74	10,02	10,31
5,50	12,90	6,74	7,04	7,33	7,62	7,92	8,21	8,50	8,79	9,09	9,38	9,67	9,97	10,26	10,55	10,85

a) The anchor spacing and the edge distance shall not be smaller than the minimum anchor spacing s_{min} and the minimum edge distance c_{min} .

Influence of embedment depth

h _{ef} /d	4	4,5	5	6	7	8	9	10	11
f _{hef} = 0,05 · (h _{ef} / d) ^{1,68}	0,51	0,63	0,75	1,01	1,31	1,64	2,00	2,39	2,81
h _{ef} /d	12	13	14	15	16	17	18	19	20
f _{hef} = 0,05 · (h _{ef} / d) ^{1,68}	3,25	3,72	4,21	4,73	5,27	5,84	6,42	7,04	7,67

Influence of edge distance ^{a)}

c/d	4	6	8	10	15	20	30	40
f _c = (d / c) ^{0,19}	0,77	0,71	0,67	0,65	0,60	0,57	0,52	0,50

a) The edge distance shall not be smaller than the minimum edge distance c_{min} .

Combined TENSION and SHEAR loading

For combined tension and shear loading see section "Anchor Design".

Precalculated values – design resistance values

All data applies to:

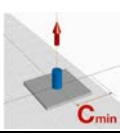
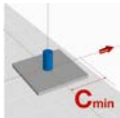
- non-cracked concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$
- temperature range I (see service temperature range)
- minimum thickness of base material
- no effects of dense reinforcement

Recommended loads can be calculated by dividing the design resistance by an overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

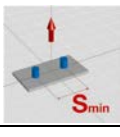
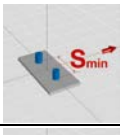

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - minimum embedment depth

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = h_{ef,min}$ [mm]		60	60	70	80	90	100	110	120	
Base material thickness $h = h_{min}$ [mm]		100	100	100	116	138	156	170	190	
Tensile N_{Rd}: single anchor, no edge effects										
Non-cracked concrete										
	HIT-V 5.8 / 8.8									
	HIT-V-R / -HCR [kN]	11,7	13,0	16,4	24,1	28,7	33,7	38,8	36,9	
Cracked concrete										
	HIT-V 5.8 / 8.8									
	HIT-V-R / -HCR [kN]	-	6,9	9,7	16,1	20,5	24,0	-	-	
Shear V_{Rd}: single anchor, no edge effects, without lever arm										
Non-cracked concrete										
	HIT-V 5.8	[kN]	7,2	12,0	16,8	31,2	48,8	67,3	77,7	88,5
	HIT-V 8.8	[kN]	12,0	18,4	27,2	48,2	57,5	67,3	77,7	88,5
	HIT-V-R	[kN]	8,3	12,8	19,2	35,3	55,1	67,3	48,3	58,8
	HIT-V-HCR	[kN]	12,0	18,4	27,2	48,2	57,5	67,3	77,7	88,5
Cracked concrete										
	HIT-V 5.8	[kN]	-	12,0	16,8	31,2	41,0	48,0	-	-
	HIT-V 8.8	[kN]	-	13,8	19,4	32,2	41,0	48,0	-	-
	HIT-V-R	[kN]	-	12,8	19,2	32,2	41,0	48,0	-	-
	HIT-V-HCR	[kN]	-	13,8	19,4	32,2	41,0	48,0	-	-

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - minimum embedment depth

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = h_{ef,min}$ [mm]	60	60	70	80	90	100	110	120	
Base material thickness $h = h_{min}$ [mm]	100	100	100	116	138	156	170	190	
Edge distance $c = c_{min}$ [mm]	40	50	60	80	100	120	135	150	
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)								
	Non-cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	7,1	8,5	9,7	15,4	20,3	25,3	29,4	28,9
	Cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	-	4,7	6,6	12,1	16,4	20,3	-	-
		Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm							
Non-cracked concrete									
HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]		3,5	4,9	6,6	10,2	14,1	18,3	21,8	25,9
Cracked concrete									
HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]		-	3,5	4,7	7,2	10,0	12,9	-	-

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - minimum embedment depth (load values are valid for single anchor)

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = h_{ef,min}$ [mm]	60	60	70	80	90	100	110	120	
Base material thickness $h = h_{min}$ [mm]	100	100	100	116	138	156	170	190	
Spacing $s = s_{min}$ [mm]	40	50	60	80	100	120	135	150	
	Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)								
	Non-cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	7,4	8,2	10,0	15,1	18,7	22,5	26,0	25,0
	Cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	-	4,9	6,7	10,7	13,3	16,0	-	-
		Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$), without lever arm							
Non-cracked concrete									
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	39,4	47,1	54,7	62,7
HIT-V 8.8 [kN]		12,0	18,4	25,4	32,1	39,4	47,1	54,7	62,7
HIT-V-R [kN]		8,3	12,8	19,2	32,1	39,4	47,1	48,3	58,8
HIT-V-HCR [kN]		12,0	18,4	25,4	32,1	39,4	47,1	54,7	62,7
	Cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	-	8,8	12,4	21,4	28,1	33,6	-	-

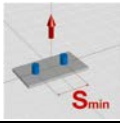
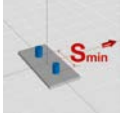
Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - typical embedment depth

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]		80	90	110	125	170	210	240	270	
Base material thickness $h = h_{min}$ [mm]		110	120	140	161	218	266	300	340	
	Tensile N_{Rd}: single anchor, no edge effects									
	Non-cracked concrete									
	HIT-V 5.8 [kN]	12,0	19,3	28,0	47,1	74,6	102,5	125,2	120,2	
	HIT-V 8.8 [kN]	15,6	22,0	32,3	47,1	74,6	102,5	125,2	120,2	
	HIT-V-R [kN]	13,9	21,9	31,6	47,1	74,6	102,5	80,4	98,3	
	HIT-V-HCR [kN]	15,6	22,0	32,3	47,1	74,6	102,5	125,2	120,2	
	Cracked concrete									
	HIT-V 5.8 / 8.8 [kN]	-	10,4	15,2	25,1	42,7	63,3	-	-	
	HIT-V-R / -HCR [kN]	-	10,4	15,2	25,1	42,7	63,3	-	-	
		Shear V_{Rd}: single anchor, no edge effects, without lever arm								
Non-cracked concrete										
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0	
HIT-V 8.8 [kN]		12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2	
HIT-V-R [kN]		8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8	
HIT-V-HCR [kN]		12,0	18,4	27,2	50,4	78,4	70,9	92,0	112,0	
Cracked concrete										
HIT-V 5.8 [kN]		-	12,0	16,8	31,2	48,8	70,4	-	-	
HIT-V 8.8 [kN]		-	18,4	27,2	50,3	78,4	112,8	-	-	
HIT-V-R [kN]		-	12,8	19,2	35,3	55,1	79,5	-	-	
HIT-V-HCR [kN]	-	18,4	27,2	50,3	78,4	70,9	-	-		

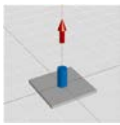
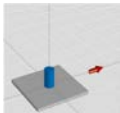
Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - typical embedment depth

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]		80	90	110	125	170	210	240	270	
Base material thickness $h = h_{min}$ [mm]		110	120	140	161	218	266	300	340	
Edge distance $c = c_{min}$ [mm]		40	50	60	80	100	120	135	150	
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)									
	Non-cracked concrete									
	HIT-V 5.8 / 8.8 [kN]	8,6	11,6	15,5	23,7	36,6	49,8	60,6	60,0	
	HIT-V-R / -HCR [kN]	8,6	11,6	15,5	23,7	36,6	49,8	60,6	60,0	
	Cracked concrete									
	HIT-V 5.8 / 8.8 [kN]	-	5,8	8,4	14,8	24,4	36,9	-	-	
HIT-V-R / -HCR [kN]	-	5,8	8,4	14,8	24,4	36,9	-	-		
	Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm									
	Non-cracked concrete									
	HIT-V 5.8 / 8.8 [kN]	3,7	5,3	7,3	11,5	17,2	23,6	29,0	34,8	
	HIT-V-R / -HCR [kN]	3,7	5,3	7,3	11,5	17,2	23,6	29,0	34,8	
	Cracked concrete									
	HIT-V 5.8 / 8.8 [kN]	-	3,8	5,2	8,1	12,2	16,7	-	-	
HIT-V-R / -HCR [kN]	-	3,8	5,2	8,1	12,2	16,7	-	-		

**Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - typical embedment depth
(load values are valid for single anchor)**

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]		80	90	110	125	170	210	240	270
Base material thickness $h = h_{min}$ [mm]		110	120	140	161	218	266	300	340
Spacing s [mm]		40	50	60	80	100	120	135	150
	Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)								
	Non-cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	9,9	13,4	18,1	26,9	42,2	57,7	70,4	69,9
	Cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	-	7,1	10,3	16,5	27,3	39,9	-	-
		Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$), without lever arm							
Non-cracked concrete									
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]		12,0	18,4	27,2	50,4	78,4	112,8	147,2	170,9
HIT-V-R [kN]		8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
HIT-V-HCR [kN]		12,0	18,4	27,2	50,4	78,4	70,9	92,0	112,0
Cracked concrete									
HIT-V 5.8 [kN]		-	12,0	16,8	30,5	48,8	70,4	-	-
HIT-V 8.8 [kN]		-	12,3	18,0	30,5	51,1	75,4	-	-
HIT-V-R [kN]		-	12,3	18,0	30,5	51,1	75,4	-	-
HIT-V-HCR [kN]	-	12,3	18,0	30,5	51,1	70,9	-	-	

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - embedment depth = $12 d^a$)

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
Embedment depth $h_{ef} = 12 d^a$) [mm]		96	120	144	192	240	288	324	360
Base material thickness $h = h_{min}$ [mm]		126	150	174	228	288	344	384	430
	Tensile N_{Rd}: single anchor, no edge effects								
	Non-cracked concrete								
	HIT-V 5.8 [kN]	12,0	19,3	28,0	52,7	82,0	118,0	153,3	160,2
	HIT-V 8.8 [kN]	18,8	29,3	42,2	83,6	120,6	159,2	183,2	160,2
	HIT-V-R [kN]	13,9	21,9	31,6	58,8	92,0	132,1	80,4	98,3
	HIT-V-HCR [kN]	18,8	29,3	42,2	83,6	120,6	117,6	152,9	160,2
	Shear V_{Rd}: single anchor, no edge effects, without lever arm								
	Non-cracked concrete								
	HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
	HIT-V 8.8 [kN]	12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2
	HIT-V-R [kN]	8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
	HIT-V-HCR [kN]	12,0	18,4	27,2	50,4	78,4	70,9	92,0	112,0
	Cracked concrete								
	HIT-V 5.8 [kN]	-	12,0	16,8	31,2	48,8	70,4	-	-
	HIT-V 8.8 [kN]	-	18,4	27,2	50,4	78,4	112,8	-	-
	HIT-V-R [kN]	-	12,8	19,2	35,3	55,1	79,5	-	-
HIT-V-HCR [kN]	-	18,4	27,2	50,4	78,4	70,9	-	-	

a) d = element diameter

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - embedment depth = $12 d^a$

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = 12 d^a$ [mm]	96	120	144	192	240	288	324	360	
Base material thickness $h = h_{min}$ [mm]	126	150	174	228	288	344	384	430	
Edge distance $c = c_{min}$ [mm]	40	50	60	80	100	120	135	150	
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)								
	Non-cracked concrete								
	HIT-V 5.8 [kN]	10,4	16,2	21,7	40,1	56,0	73,6	87,8	85,7
	HIT-V 8.8 [kN]	10,4	16,2	21,7	40,1	56,0	73,6	87,8	85,7
	HIT-V-R [kN]	10,4	16,2	21,7	40,1	56,0	73,6	80,4	85,7
	HIT-V-HCR [kN]	10,4	16,2	21,7	40,1	56,0	73,6	87,8	85,7
	Cracked concrete								
	HIT-V 5.8 / 8.8 [kN]	-	7,6	11,0	21,7	34,4	50,6	-	-
	HIT-V-R / -HCR [kN]	-	7,6	11,0	21,7	34,4	50,6	-	-
		Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm							
Non-cracked concrete									
HIT-V 5.8 / 8.8 [kN]		3,9	5,7	7,8	12,9	18,9	25,9	31,8	38,1
HIT-V-R / -HCR [kN]		3,9	5,7	7,8	12,9	18,9	25,9	31,8	38,1
Cracked concrete									
HIT-V 5.8 / 8.8 [kN]		-	4,0	5,5	9,1	13,4	18,4	-	-
HIT-V-R / -HCR [kN]		-	4,0	5,5	9,1	13,4	18,4	-	-






a) d = element diameter

Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - embedment depth = $12 d^a$
(load values are valid for single anchor)

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = 12 d^a$ [mm]	96	120	144	192	240	288	324	360	
Base material thickness $h = h_{min}$ [mm]	126	150	174	228	288	344	384	430	
Spacing $s = s_{min}$ [mm]	40	50	60	80	100	120	135	150	
	Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)								
	Non-cracked concrete								
	HIT-V 5.8 [kN]	12,0	18,5	26,0	48,9	68,4	89,9	107,2	103,8
	HIT-V 8.8 [kN]	12,1	18,5	26,0	48,9	68,4	89,9	107,2	103,8
	HIT-V-R [kN]	12,1	18,5	26,0	48,9	68,4	89,9	80,4	98,3
	HIT-V-HCR [kN]	12,1	18,5	26,0	48,9	68,4	89,9	107,2	103,8
	Cracked concrete								
	HIT-V 5.8 / 8.8 [kN]	-	9,6	13,6	25,8	39,8	56,7	-	-
	HIT-V-R / -HCR [kN]	-	9,6	13,6	25,8	39,8	56,7	-	-
		Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$), without lever arm							
Non-cracked concrete									
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]		12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2
HIT-V-R [kN]		8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
HIT-V-HCR [kN]		12,0	18,4	27,2	50,4	78,4	70,9	92,0	112,0
Cracked concrete									
HIT-V 5.8 [kN]		-	12,0	16,8	31,2	48,8	70,4	-	-
HIT-V 8.8 [kN]		-	15,7	22,7	44,0	68,7	98,9	-	-
HIT-V-R [kN]		-	12,8	19,2	35,3	55,1	79,5	-	-
HIT-V-HCR [kN]	-	15,7	22,7	44,0	68,7	70,9	-	-	

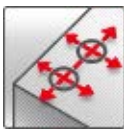
a) d = element diameter

Hilti HIT-HY 150 MAX with HIS-(R)N

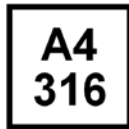
Injection mortar system		Benefits
    	<p>Hilti HIT-HY 150 MAX 330 ml foil pack (also available as 500 ml and 1400 ml foil pack)</p> <p>Static mixer</p> <p>Internal threaded sleeve HIS-N HIS-RN</p>	<ul style="list-style-type: none"> - suitable for non-cracked concrete C 20/25 to C 50/60 - suitable for dry and water saturated concrete - high loading capacity - rapid curing - small edge distance and anchor spacing possible - corrosion resistant - in service temperature range up to 120°C short term/72°C long term - manual cleaning for anchor size M8 and M10



Concrete



Small edge distance and spacing



Corrosion resistance



European Technical Approval



CE conformity



PROFIS Anchor design software

Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval ^{a)}	DIBt, Berlin	ETA-08/0352 / 2010-04-01

a) All data given in this section according ETA-08/0352 issue 2010-04-01

Basic loading data (for a single anchor)

All data in this section applies to

- Correct setting (See setting instruction)
- No edge distance and spacing influence
- Steel failure
- Base material thickness, as specified in the table
- One anchor material, as specified in the tables
- Concrete C 20/25, $f_{ck,cube} = 25 \text{ N/mm}^2$
- Temperate range I
(min. base material temperature -40°C, max. long term/short term base material temperature: +24°C/40°C)
- Installation temperature range -10°C to +40°C

For details see Simplified design method

Embedment depth and base material thickness for the basic loading data. Mean ultimate resistance, characteristic resistance, design resistance, recommended loads.

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Embedment depth	h_{ef} [mm]	90	110	125	170	205
Base material thickness	h [mm]	120	150	170	230	270

Mean ultimate resistance: non-cracked concrete C 20/25 , anchor HIS-N

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Tensile $N_{Ru,m}$	HIS-N [kN]	26,3	48,3	70,4	123,9	114,5
Shear $V_{Ru,m}$	HIS-N [kN]	13,7	24,2	41,0	62,0	57,8

Characteristic resistance: non-cracked concrete C 20/25 , anchor HIS-N

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Tensile N_{Rk}	HIS-N [kN]	25,0	46,0	67,0	95,0	109,0
Shear V_{Rk}	HIS-N [kN]	13,0	23,0	39,0	59,0	55,0

Design resistance: non-cracked concrete C 20/25 , anchor HIS-N

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Tensile N_{Rd}	HIS-N [kN]	17,5	26,7	40,0	62,2	74,1
Shear V_{Rd}	HIS-N [kN]	10,4	18,4	26,0	39,3	36,7

Recommended loads ^{a)}: non-cracked concrete C 20/25 , anchor HIS-N

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Tensile N_{rec}	HIS-N [kN]	12,5	19,8	31,9	45,2	53,0
Shear V_{rec}	HIS-N [kN]	7,4	13,1	18,6	28,1	26,2

a) With overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Service temperature range

Hilti HIT-HY 150 MAX injection mortar may be applied in the temperature ranges given below. An elevated base material temperature may lead to a reduction of the design bond resistance.

Temperature range	Base material temperature	Maximum long term base material temperature	Maximum short term base material temperature
Temperature range I	-40 °C to +40 °C	+24 °C	+40 °C
Temperature range II	-40 °C to +80 °C	+50 °C	+80 °C
Temperature range III	-40 °C to +120 °C	+72 °C	+120 °C

Max short term base material temperature

Short-term elevated base material temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Max long term base material temperature

Long-term elevated base material temperatures are roughly constant over significant periods of time.

Materials

Mechanical properties of HIS-(R)N

Anchor size			M8x90	M10x110	M12x125	M16x170	M20x205
Nominal tensile strength f_{uk}	HIS-N	[N/mm ²]	490	490	460	460	460
	Screw 8.8	[N/mm ²]	800	800	800	800	800
	HIS-RN	[N/mm ²]	700	700	700	700	700
	Screw A4-70	[N/mm ²]	700	700	700	700	700
Yield strength f_{yk}	HIS-N	[N/mm ²]	410	410	375	375	375
	Screw 8.8	[N/mm ²]	640	640	640	640	640
	HIS-RN	[N/mm ²]	350	350	350	350	350
	Screw A4-70	[N/mm ²]	450	450	450	450	450
Stressed cross-section A_s	HIS-(R)N	[mm ²]	51,5	108,0	169,1	256,1	237,6
	Screw	[mm ²]	36,6	58	84,3	157	245
Moment of resistance W	HIS-(R)N	[mm ³]	145	430	840	1595	1543
	Screw	[mm ³]	31,2	62,3	109	277	541

Material quality

Part	Material
Internal threaded sleeve ^{a)} HIS-N	C-steel 1.0718, Steel galvanized $\geq 5\mu\text{m}$
Internal threaded sleeve ^{a)} HIS-RN	Stainless steel 1.4401 and 1.4571

- a) related fastening screw: strength class 8.8, A5 > 8% Ductile
steel galvanized $\geq 5\mu\text{m}$
- b) related fastening screw: strength class 70, A5 > 8% Ductile
stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362

Anchor dimensions

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Internal threaded sleeve HIS-N / HIS-RN						
Embedment depth	h_{ef} [mm]	80	90	110	125	170

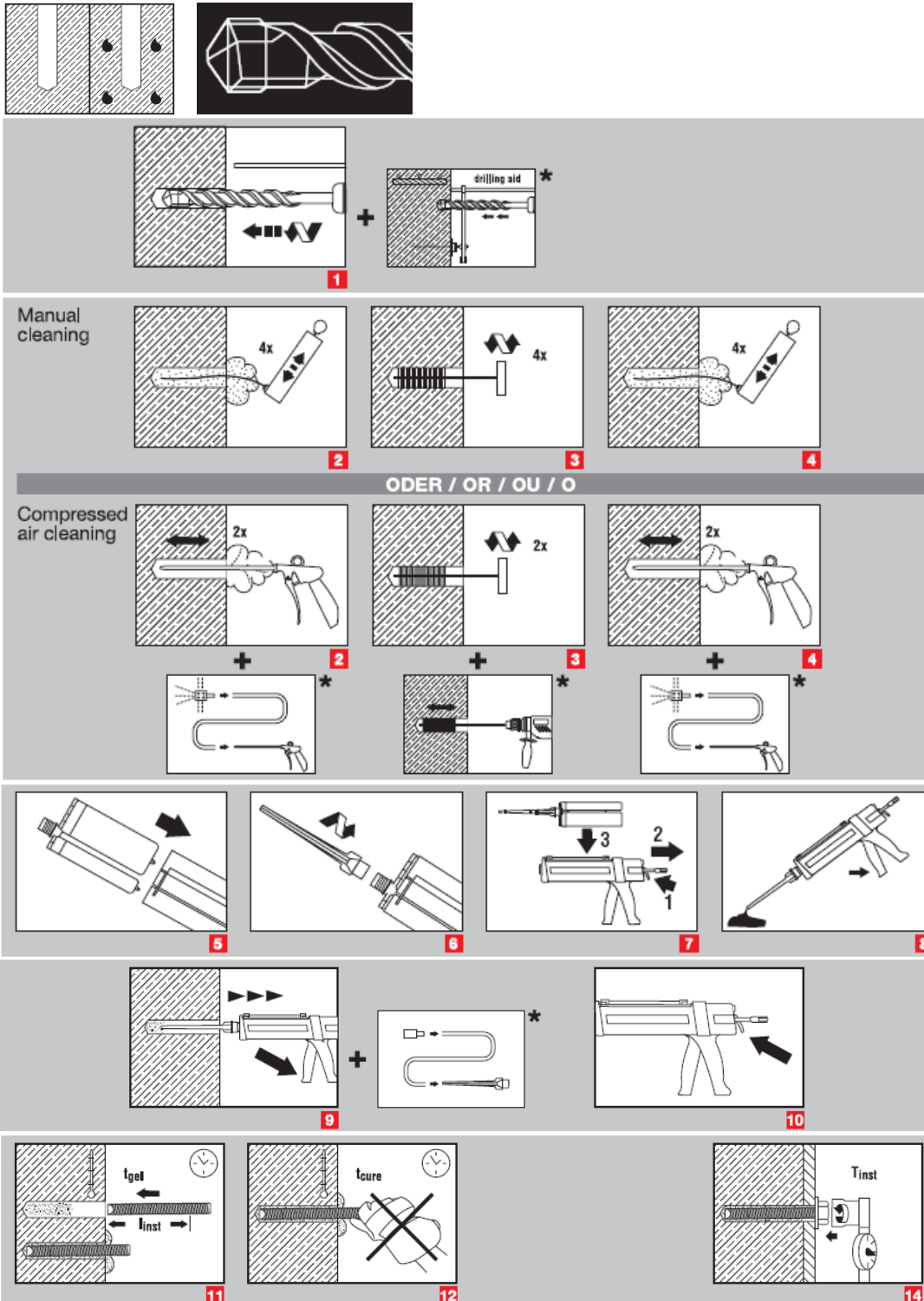
Setting

installation equipment

Anchor size	M8x90	M10x110	M12x125	M16x170	M20x205
Rotary hammer	TE 2 – TE 16		TE 40 – TE 70		
Other tools	compressed air gun or blow out pump, set of cleaning brushes, dispenser				

Setting instruction

Dry and water-saturated concrete, hammer drilling



a)

b)

a) Note: Manual cleaning for HIS-(R)N M8 and HIS-(R)N M10 only!

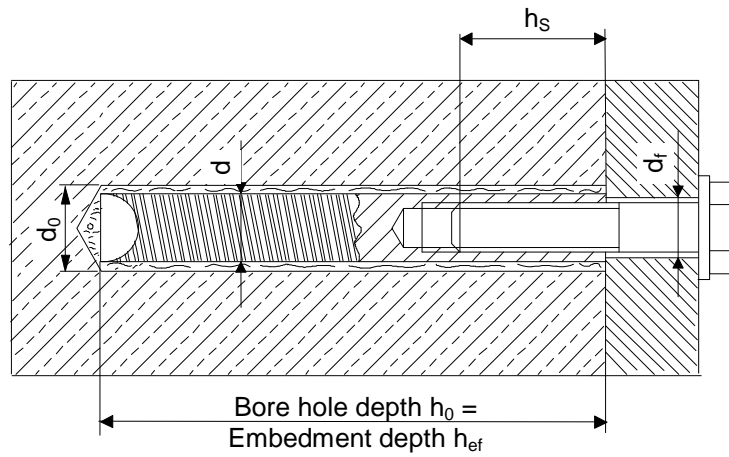
b) Note: Extension and piston plug needed for overhead installation!

For detailed information on installation see instruction for use given with the package of the product.

Working time, Curing time

Temperature of the base material T_{BM}	Working time t_{gel}	Curing time t_{cure}
$-10\text{ °C} \leq T_{BM} < -5\text{ °C}$	180 min	12 h
$-5\text{ °C} \leq T_{BM} < 0\text{ °C}$	40 min	4 h
$0\text{ °C} \leq T_{BM} < 5\text{ °C}$	20 min	2 h
$5\text{ °C} \leq T_{BM} < 20\text{ °C}$	8 min	1 h
$20\text{ °C} \leq T_{BM} < 30\text{ °C}$	5 min	30 min
$30\text{ °C} \leq T_{BM} \leq 40\text{ °C}$	2 min	30 min

Setting details



Anchor size			M8x90	M10x110	M12x125	M16x170	M20x205	
Nominal diameter of drill bit	d_0	[mm]	14	18	22	28	32	
Diameter of element	d	[mm]	12,5	16,5	20,5	25,4	27,6	
Effective anchorage and drill hole depth	h_{ef}	[mm]	90	110	125	170	205	
Minimum base material thickness	h_{min}	[mm]	120	150	170	230	270	
Diameter of clearance hole in the fixture	d_f	[mm]	9	12	14	18	22	
Thread engagement length; min - max	h_s	[mm]	8-20	10-25	12-30	16-40	20-50	
Torque moment ^{a)}	T_{max}	[Nm]	10	20	40	80	150	
Minimum spacing	s_{min}	[mm]	40	45	55	65	90	
Minimum edge distance	c_{min}	[mm]	40	45	55	65	90	
Critical spacing for splitting failure	$s_{cr,sp}$	[mm]	$2 c_{cr,sp}$					
Critical edge distance for splitting failure ^{c)}	$c_{cr,sp}$	[mm]	$1,0 \cdot h_{ef}$ for $h / h_{ef} \geq 2,0$					
			$4,6 h_{ef} - 1,8 h$ for $2,0 > h / h_{ef} > 1,3$					
			$2,26 h_{ef}$ for $h / h_{ef} \leq 1,3$					
Critical spacing for concrete cone failure	$s_{cr,N}$	[mm]	$2 c_{cr,N}$					
Critical edge distance for concrete cone failure ^{c)}	$c_{cr,N}$	[mm]	$1,5 h_{ef}$					

For spacing (or edge distance) smaller than critical spacing (or critical edge distance) the design loads have to be reduced.

- Maximum recommended torque moment to avoid splitting failure during installation with minimum spacing and/or edge distance.
- h : base material thickness ($h \geq h_{min}$), h_{ef} : embedment depth
- The critical edge distance for concrete cone failure depends on the embedment depth h_{ef} and the design bond resistance. The simplified formula given in this table is on the safe side.

Simplified design method

Simplified version of the design method according ETAG 001, TR 029. Design resistance according data given in ETA-08/0352, issue 2010-04-01.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing
- Valid for a group of two anchors. (The method may also be applied for anchor groups with more than two anchors or more than one edge distance. The influencing factors must then be considered for each edge distance and spacing. The calculated design loads are then on the save side: They will be lower than the exact values according ETAG 001, TR 029. To avoid this, it is recommended to use the anchor design software PROFIS anchor)

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

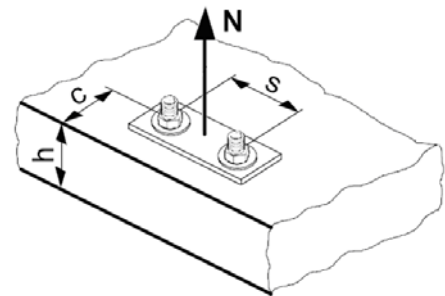
The values are valid for one anchor.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

TENSION loading

The design tensile resistance is the lower value of

- Steel resistance: $N_{Rd,s}$
- Combined pull-out and concrete cone resistance:
 $N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$
- Concrete cone resistance: $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$
- Concrete splitting resistance (only non-cracked concrete):
 $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$



Basic design tensile resistance

Design steel resistance $N_{Rd,s}$

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
$N_{Rd,s}$	HIS-N [kN]	17,5	30,7	44,7	80,3	74,1
	HIS-RN [kN]	13,9	21,9	31,6	58,8	69,2

Design combined pull-out and concrete cone resistance

$$N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$$

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Embedment depth	h_{ef} [mm]	90	110	125	170	205
$N_{Rd,p}^0$	Temperature range I [kN]	19,4	27,8	50,0	63,3	76,7
$N_{Rd,p}^0$	Temperature range II [kN]	16,7	27,8	40,0	63,3	63,3
$N_{Rd,p}^0$	Temperature range III [kN]	11,1	16,7	26,7	40,0	40,0

Design concrete cone resistance $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$

Design splitting resistance $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$

Anchor size		M8	M10	M12	M16	M20
$N_{Rd,c}^0$	[kN]	24,0	32,4	47,1	74,6	98,8

Influencing factors

Influence of concrete strength on combined pull-out and concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_{B,p} = (f_{ck,cube}/25N/mm^2)^{0,10}$ ^{a)}	1,00	1,02	1,04	1,06	1,07	1,08	1,09

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of embedment depth on combined pull-out and concrete cone resistance

$f_{h,p} = 1$

Influence of concrete strength on concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{0,5}$ ^{a)}	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of edge distance ^{a)}

$c/c_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$c/c_{cr,sp}$										
$f_{1,N} = 0,7 + 0,3 \cdot c/c_{cr,N} \leq 1$	0,73	0,76	0,79	0,82	0,85	0,88	0,91	0,94	0,97	1
$f_{1,sp} = 0,7 + 0,3 \cdot c/c_{cr,sp} \leq 1$										
$f_{2,N} = 0,5 \cdot (1 + c/c_{cr,N}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{2,sp} = 0,5 \cdot (1 + c/c_{cr,sp}) \leq 1$										

a) The edge distance shall not be smaller than the minimum edge distance c_{min} . These influencing factors must be considered for every edge distance smaller than the critical edge distance.

Influence of anchor spacing ^{a)}

$s/s_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$s/s_{cr,sp}$										
$f_{3,N} = 0,5 \cdot (1 + s/s_{cr,N}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{3,sp} = 0,5 \cdot (1 + s/s_{cr,sp}) \leq 1$										

a) The anchor spacing shall not be smaller than the minimum anchor spacing s_{min} . This influencing factor must be considered for every anchor spacing.

Influence of embedment depth on concrete cone resistance

$f_{h,N} = 1$

Influence of reinforcement

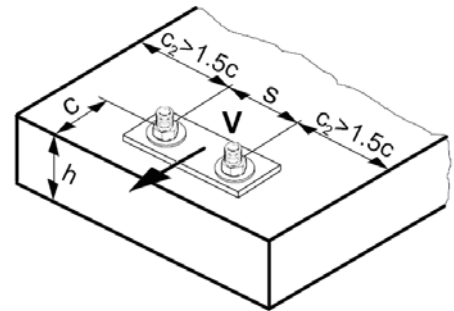
h_{ef} [mm]	40	50	60	70	80	90	≥ 100
$f_{re,N} = 0,5 + h_{ef}/200mm \leq 1$	0,7 ^{a)}	0,75 ^{a)}	0,8 ^{a)}	0,85 ^{a)}	0,9 ^{a)}	0,95 ^{a)}	1

a) This factor applies only for dense reinforcement. If in the area of anchorage there is reinforcement with a spacing ≥ 150 mm (any diameter) or with a diameter ≤ 10 mm and a spacing ≥ 100 mm, then a factor $f_{re,N} = 1$ may be applied.

SHEAR loading

The design shear resistance is the lower value of

- Steel resistance: $V_{Rd,s}$
- Concrete pryout resistance: $V_{Rd,cp} = k \cdot \text{lower value of } N_{Rd,p} \text{ and } N_{Rd,c}$
- Concrete edge resistance: $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_{\beta} \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$



Basic design shear resistance

Design steel resistance $V_{Rd,s}$

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
$V_{Rd,s}$	HIS-N [kN]	10,4	18,4	26,0	39,3	36,7
	HIS-RN [kN]	8,3	12,8	19,2	35,3	41,5

Design concrete pryout resistance $V_{Rd,cp} = \text{lower value}^a$ of $k \cdot N_{Rd,p}$ and $k \cdot N_{Rd,c}$

$$k = 2 \text{ for } h_{ef} \geq 60 \text{ mm}$$

- a) $N_{Rd,p}$: Design combined pull-out and concrete cone resistance
 $N_{Rd,c}$: Design concrete cone resistance

Design concrete edge resistance $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_{\beta} \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$

Anchor size		M8	M10	M12	M16	M20
Non-cracked concrete						
$V_{Rd,c}^0$ [kN]		12,4	19,6	28,2	40,2	46,2

Influencing factors

Influence of concrete strength

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{1/2}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

- a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of angle between load applied and the direction perpendicular to the free edge

Angle β	0°	10°	20°	30°	40°	50°	60°	70°	80°	≥ 90°
$f_{\beta} = \frac{1}{\sqrt{(\cos \alpha_v)^2 + \left(\frac{\sin \alpha_v}{2,5}\right)^2}}$	1	1,01	1,05	1,13	1,24	1,40	1,64	1,97	2,32	2,50

Influence of base material thickness

h/c	0,15	0,3	0,45	0,6	0,75	0,9	1,05	1,2	1,35	≥ 1,5
$f_h = \{h/(1,5 \cdot c)\}^{1/2} \leq 1$	0,32	0,45	0,55	0,63	0,71	0,77	0,84	0,89	0,95	1,00

Influence of anchor spacing and edge distance ^{a)} for concrete edge resistance: f_4

$$f_4 = (c/h_{ef})^{1,5} \cdot (1 + s / [3 \cdot c]) \cdot 0,5$$

c/h _{ef}	Single anchor	Group of two anchors s/h _{ef}														
		0,75	1,50	2,25	3,00	3,75	4,50	5,25	6,00	6,75	7,50	8,25	9,00	9,75	10,50	11,25
0,50	0,35	0,27	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35
0,75	0,65	0,43	0,54	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65
1,00	1,00	0,63	0,75	0,88	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00
1,25	1,40	0,84	0,98	1,12	1,26	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40
1,50	1,84	1,07	1,22	1,38	1,53	1,68	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84
1,75	2,32	1,32	1,49	1,65	1,82	1,98	2,15	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32
2,00	2,83	1,59	1,77	1,94	2,12	2,30	2,47	2,65	2,83	2,83	2,83	2,83	2,83	2,83	2,83	2,83
2,25	3,38	1,88	2,06	2,25	2,44	2,63	2,81	3,00	3,19	3,38	3,38	3,38	3,38	3,38	3,38	3,38
2,50	3,95	2,17	2,37	2,57	2,77	2,96	3,16	3,36	3,56	3,76	3,95	3,95	3,95	3,95	3,95	3,95
2,75	4,56	2,49	2,69	2,90	3,11	3,32	3,52	3,73	3,94	4,15	4,35	4,56	4,56	4,56	4,56	4,56
3,00	5,20	2,81	3,03	3,25	3,46	3,68	3,90	4,11	4,33	4,55	4,76	4,98	5,20	5,20	5,20	5,20
3,25	5,86	3,15	3,38	3,61	3,83	4,06	4,28	4,51	4,73	4,96	5,18	5,41	5,63	5,86	5,86	5,86
3,50	6,55	3,51	3,74	3,98	4,21	4,44	4,68	4,91	5,14	5,38	5,61	5,85	6,08	6,31	6,55	6,55
3,75	7,26	3,87	4,12	4,36	4,60	4,84	5,08	5,33	5,57	5,81	6,05	6,29	6,54	6,78	7,02	7,26
4,00	8,00	4,25	4,50	4,75	5,00	5,25	5,50	5,75	6,00	6,25	6,50	6,75	7,00	7,25	7,50	7,75
4,25	8,76	4,64	4,90	5,15	5,41	5,67	5,93	6,18	6,44	6,70	6,96	7,22	7,47	7,73	7,99	8,25
4,50	9,55	5,04	5,30	5,57	5,83	6,10	6,36	6,63	6,89	7,16	7,42	7,69	7,95	8,22	8,49	8,75
4,75	10,35	5,45	5,72	5,99	6,27	6,54	6,81	7,08	7,36	7,63	7,90	8,17	8,45	8,72	8,99	9,26
5,00	11,18	5,87	6,15	6,43	6,71	6,99	7,27	7,55	7,83	8,11	8,39	8,66	8,94	9,22	9,50	9,78
5,25	12,03	6,30	6,59	6,87	7,16	7,45	7,73	8,02	8,31	8,59	8,88	9,17	9,45	9,74	10,02	10,31
5,50	12,90	6,74	7,04	7,33	7,62	7,92	8,21	8,50	8,79	9,09	9,38	9,67	9,97	10,26	10,55	10,85

a) The anchor spacing and the edge distance shall not be smaller than the minimum anchor spacing s_{min} and the minimum edge distance c_{min} .

Influence of embedment depth

Anchor size	M8	M10	M12	M16	M20
$f_{hef} =$	1,38	1,21	1,04	1,22	1,45

Influence of edge distance ^{a)}

c/d	4	6	8	10	15	20	30	40
$f_c = (d / c)^{0,19}$	0,77	0,71	0,67	0,65	0,60	0,57	0,52	0,50

a) The edge distance shall not be smaller than the minimum edge distance c_{min} .

Combined TENSION and SHEAR loading

For combined tension and shear loading see section "Anchor Design".

Precalculated values – design resistance values

All data applies to:

- non-cracked concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$
- temperature range I (see service temperature range)
- minimum thickness of base material
- no effects of dense reinforcement

Recommended loads can be calculated by dividing the design resistance by an overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Design resistance: non-cracked- concrete C 20/25

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205	
Embedment depth	h_{ef} [mm]	90	110	125	170	205	
Base material thickness	$h = h_{min}$ [mm]	120	150	170	230	270	
	Tensile N_{Rd}: single anchor, no edge effects						
	HIS-N	[kN]	17,5	27,8	44,7	63,3	74,1
	HIS-RN	[kN]	13,9	21,9	31,6	58,8	69,2
	Shear V_{Rd}: single anchor, no edge effects, without lever arm						
	HIS-N	[kN]	10,4	18,4	26,0	39,3	36,7
	HIS-RN	[kN]	8,3	12,8	19,2	35,3	41,5






Design resistance: non-cracked- concrete C 20/25

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205	
Embedment depth	h_{ef} [mm]	90	110	125	170	205	
Base material thickness	$h = h_{min}$ [mm]	120	150	170	230	270	
Edge distance	$c = c_{min}$ [mm]	40	45	55	65	90	
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)						
	HIS-N	[kN]	9,9	13,8	21,6	31,2	41,7
	HIS-RN	[kN]	9,9	13,8	21,6	31,2	41,7
	Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm						
	HIS-N	[kN]	4,2	5,5	7,6	10,8	17,2
	HIS-RN	[kN]	4,2	5,5	7,6	10,8	17,2

Design resistance: non-cracked- concrete C 20/25

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205	
Embedment depth	h_{ef} [mm]	90	110	125	170	205	
Base material thickness	$h = h_{min}$ [mm]	120	150	170	230	270	
Spacing	$s = s_{min}$ [mm]	40	45	55	65	90	
	Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)						
	HIS-N	[kN]	11,9	16,6	25,9	37,9	48,4
	HIS-RN	[kN]	11,9	16,6	25,9	37,9	48,4
	Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$), without lever arm						
	HIS-N	[kN]	10,4	18,4	26,0	39,3	36,7
	HIS-RN	[kN]	8,3	12,8	19,2	35,3	41,5

Hilti HIT-HY 150 MAX with rebar

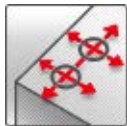
Injection mortar system		Benefits
    	<p>Hilti HIT-HY 150 MAX 330 ml foil pack (also available as 500 ml and 1400 ml foil pack)</p> <p>Static mixer</p> <p>rebar BSt 500 S</p>	<ul style="list-style-type: none"> - suitable for non-cracked and cracked concrete C 20/25 to C 50/60 - suitable for dry and water saturated concrete - high loading capacity - rapid curing - small edge distance and anchor spacing possible - large diameter applications - in service temperature range up to 120°C short term/72°C long term - manual cleaning for anchor size Ø8 to Ø14 and embedment depth $h_{ef} \leq 10d$ for non-cracked concrete - embedment depth range: from 60 ... 160 mm for Ø8 to 100 ... 500 mm for Ø25



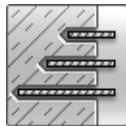
Concrete



Tensile zone



Small edge distance and spacing



Variable embedment depth



European Technical Approval



CE conformity



PROFIS Anchor design software

Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval ^{a)}	DIBt, Berlin	ETA-08/0352 / 2010-04-01

a) All data given in this section according ETA-08/0352 issue 2010-04-01.

Basic loading data (for a single anchor)

All data in this section applies to

- Correct setting (See setting instruction)
- No edge distance and spacing influence
- Steel failure
- Base material thickness, as specified in the table
- One typical embedment depth, as specified in the table
- Anchor material: rebar BSt 500 S
- Concrete C 20/25, $f_{ck,cube} = 25 \text{ N/mm}^2$
- Temperate range I
(min. base material temperature -40°C, max. long term/short term base material temperature: +24°C/40°C)
- Installation temperature range -10°C to +40°C

For details see Simplified design method

Embedment depth ^{a)} and base material thickness for the basic loading data. Mean ultimate resistance, characteristic resistance, design resistance, recommended loads.

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = h_{ef,typ}^b$ [mm]	80	90	110	125	145	170	210
Base material thickness h [mm]	110	120	140	165	185	220	274

a) The allowed range of embedment depth is shown in the setting details. The corresponding load values can be calculated according to the simplified design method.

b) $h_{ef,typ}$: Typical embedment depth

Mean ultimate resistance: non-cracked concrete C 20/25 , anchor BSt 500 S

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Non-cracked concrete							
Tensile $N_{Ru,m}$ BSt 500 S [kN]	25,5	35,8	52,5	69,6	92,3	135,3	204,9
Shear $V_{Ru,m}$ BSt 500 S [kN]	14,7	23,1	32,6	44,1	57,8	90,3	141,8
Cracked concrete							
Tensile $N_{Ru,m}$ BSt 500 S [kN]	-	20,7	30,4	44,0	58,3	85,5	131,9
Shear $V_{Ru,m}$ BSt 500 S [kN]	-	23,1	32,6	44,1	57,8	90,3	141,8

Characteristic resistance: non-cracked concrete C 20/25 , anchor BSt 500 S

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Non-cracked concrete							
Tensile N_{Rk} BSt 500 S [kN]	19,1	26,9	39,4	52,2	69,2	101,5	153,7
Shear V_{Rk} BSt 500 S [kN]	14,0	22,0	31,0	42,0	55,0	86,0	135,0
Cracked concrete							
Tensile N_{Rk} BSt 500 S [kN]	-	15,6	22,8	33,0	43,7	64,1	99,0
Shear V_{Rk} BSt 500 S [kN]	-	22,0	31,0	42,0	55,0	86,0	135,0

Design resistance: non-cracked concrete C 20/25 , anchor BSt 500 S

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Non-cracked concrete							
Tensile N_{Rd} BSt 500 S [kN]	10,6	14,9	21,9	29,0	46,2	67,6	85,4
Shear V_{Rd} BSt 500 S [kN]	9,3	14,7	20,7	28,0	36,7	57,3	90,0
Cracked concrete							
Tensile N_{Rd} BSt 500 S [kN]	-	10,4	15,2	22,0	29,2	42,7	55,0
Shear V_{Rd} BSt 500 S [kN]	-	14,7	20,7	28,0	36,7	57,3	90,0

Recommended loads ^{a)}: non-cracked concrete C 20/25 , anchor BSt 500 S

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Non-cracked concrete							
Tensile N_{rec} BSt 500 S [kN]	7,6	10,7	15,6	20,7	33,0	48,3	61,0
Shear V_{rec} BSt 500 S [kN]	6,7	10,5	14,8	20,0	26,2	41,0	64,3
Cracked concrete							
Tensile N_{rec} BSt 500 S [kN]	-	7,4	10,9	15,7	20,8	30,5	39,3
Shear V_{rec} BSt 500 S [kN]	-	10,5	14,8	20,0	26,2	41,0	64,3

a) With overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Service temperature range

Hilti HIT-HY 150 MAX injection mortar may be applied in the temperature ranges given below. An elevated base material temperature may lead to a reduction of the design bond resistance.

Temperature range	Base material temperature	Maximum long term base material temperature	Maximum short term base material temperature
Temperature range I	-40 °C to +40 °C	+24 °C	+40 °C
Temperature range II	-40 °C to +80 °C	+50 °C	+80 °C
Temperature range III	-40 °C to +120 °C	+72 °C	+120 °C

Max short term base material temperature

Short-term elevated base material temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Max long term base material temperature

Long-term elevated base material temperatures are roughly constant over significant periods of time.

Materials

Mechanical properties of rebar BSt 500S

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Nominal tensile strength f_{uk} BSt 500 S [N/mm ²]	550						
Yield strength f_{yk} BSt 500 S [N/mm ²]	500						
Stressed cross-section A_s BSt 500 S [mm ²]	50,3	78,5	113,1	153,9	201,1	314,2	490,9
Moment of resistance W BSt 500 S [mm ³]	50,3	98,2	169,6	269,4	402,1	785,4	1534

Material quality

Part	Material
rebar BSt 500 S	Mechanical properties according to DIN 488-1:1984 Geometry according to DIN 488-21:1986

Anchor dimensions

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
rebar BSt 500 S	rebar are available in variable length						

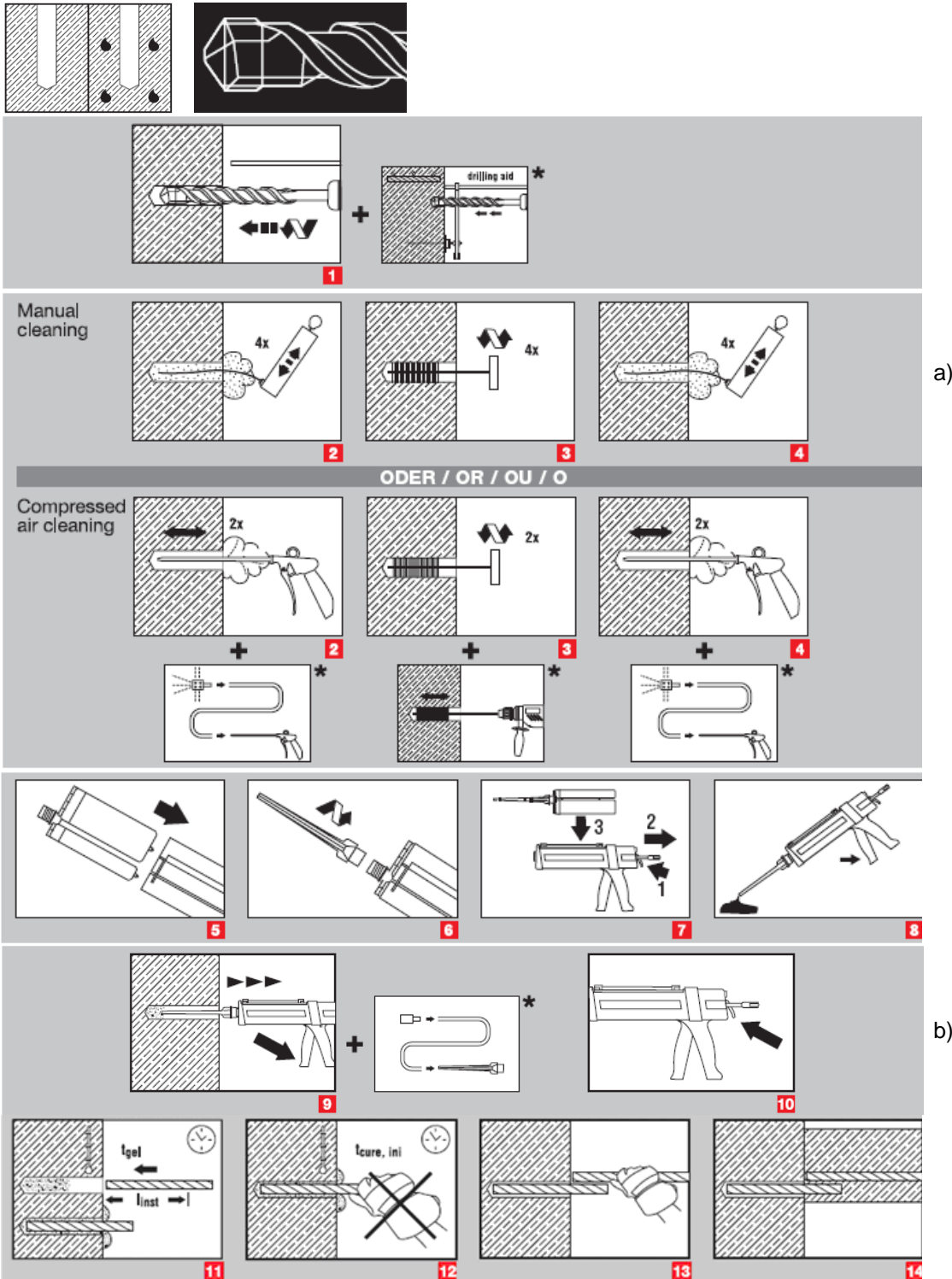
Setting

installation equipment

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Rotary hammer	TE 2 – TE 16					TE 40 – TE 70	
Other tools	compressed air gun or blow out pump, set of cleaning brushes, dispenser						

Setting instruction

Dry and water-saturated concrete, hammer drilling



a) Note: Manual cleaning for non-cracked concrete, element sizes $d \leq 14\text{mm}$ and embedment depth $h_{ef} \leq 10 d$ only!

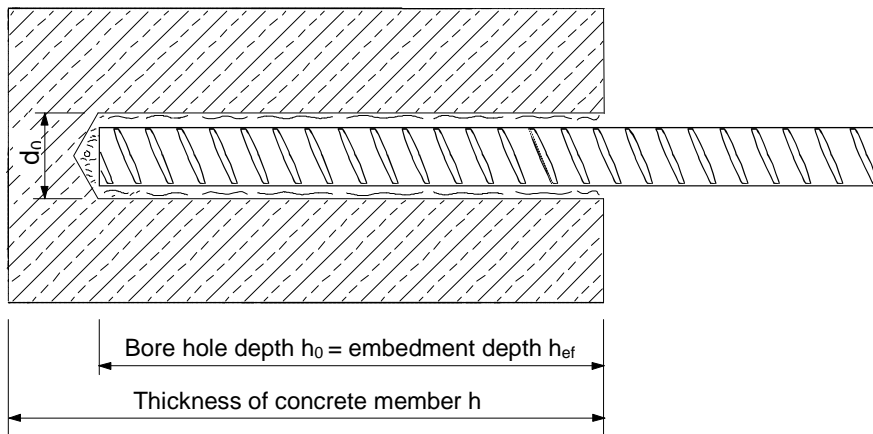
b) Note: Extension and piston plug needed for overhead installation and/or embedment depth $> 250\text{mm}$!

For detailed information on installation see instruction for use given with the package of the product.

Working time, Curing time

Temperature of the base material T_{BM}	Working time t_{gel}	Curing time t_{cure}
$-10\text{ °C} \leq T_{BM} < -5\text{ °C}$	180 min	12 h
$-5\text{ °C} \leq T_{BM} < 0\text{ °C}$	40 min	4 h
$0\text{ °C} \leq T_{BM} < 5\text{ °C}$	20 min	2 h
$5\text{ °C} \leq T_{BM} < 20\text{ °C}$	8 min	1 h
$20\text{ °C} \leq T_{BM} < 30\text{ °C}$	5 min	30 min
$30\text{ °C} \leq T_{BM} \leq 40\text{ °C}$	2 min	30 min

Setting details



Anchor size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25		
Nominal diameter of drill bit	d_0	[mm]	10-12 ^{d)}	12-14 ^{d)}	14-16 ^{d)}	18	20	25	32		
Effective embedment and drill hole depth range ^{a)} for rebar BSt 500 S	$h_{ef,min}$	[mm]	60	60	70	75	80	90	100		
	$h_{ef,max}$	[mm]	160	200	240	280	320	400	500		
Minimum base material thickness	h_{min}	[mm]	$h_{ef} + 30 \text{ mm}$ $\geq 100 \text{ mm}$			$h_{ef} + 2 d_0$					
Minimum spacing	s_{min}	[mm]	40	50	60	70	80	100	150		
Minimum edge distance	c_{min}	[mm]	40	50	60	80	100	120	150		
Critical spacing for splitting failure	$s_{cr,sp}$	[mm]	$2 c_{cr,sp}$								
Critical edge distance for splitting failure ^{c)}	$c_{cr,sp}$	[mm]	$1,0 \cdot h_{ef}$		for $h / h_{ef} \geq 2,0$						
			$4,6 h_{ef} - 1,8 h$							for $2,0 > h / h_{ef} > 1,3$	
			$2,26 h_{ef}$		for $h / h_{ef} \leq 1,3$						
Critical spacing for concrete cone failure	$s_{cr,N}$	[mm]	$2 c_{cr,N}$								
Critical edge distance for concrete cone failure ^{c)}	$c_{cr,N}$	[mm]	$1,5 h_{ef}$								

For spacing (or edge distance) smaller than critical spacing (or critical edge distance) the design loads have to be reduced.

- a) Embedment depth range: $h_{ef,min} \leq h_{ef} \leq h_{ef,max}$
- b) h : base material thickness ($h \geq h_{min}$), h_{ef} : embedment depth
- c) The critical edge distance for concrete cone failure depends on the embedment depth h_{ef} and the design bond resistance. The simplified formula given in this table is on the safe side.
- d) both given values for drill bit diameter can be used

Simplified design method

Simplified version of the design method according ETAG 001, TR 029. Design resistance according data given in ETA-08/0352, issue 2010-04-01.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing
- Valid for a group of two anchors. (The method may also be applied for anchor groups with more than two anchors or more than one edge distance. The influencing factors must then be considered for each edge distance and spacing. The calculated design loads are then on the save side: They will be lower than the exact values according ETAG 001, TR 029. To avoid this, it is recommended to use the anchor design software PROFIS anchor)

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

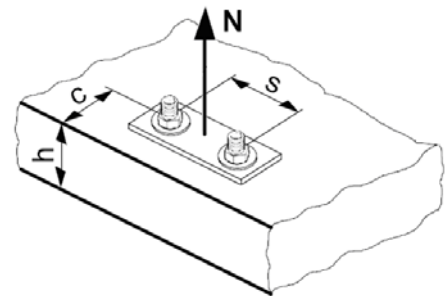
The values are valid for one anchor.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

TENSION loading

The design tensile resistance is the lower value of

- Steel resistance: $N_{Rd,s}$
- Combined pull-out and concrete cone resistance: $N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$
- Concrete cone resistance: $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$
- Concrete splitting resistance (only non-cracked concrete): $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$



Basic design tensile resistance

Design steel resistance $N_{Rd,s}$

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
$N_{Rd,s}$ BSt 500 S [kN]	20,0	30,7	44,3	60,7	79,3	123,6	192,9

Design combined pull-out and concrete cone resistance

$$N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$$

Anchor size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} =$ Typical embedment depth $h_{ef,typ}$ [mm]	80	90	110	125	145	170	210
Non-cracked concrete							
$N_{Rd,p}^0$ Temperature range I [kN]	10,6	14,9	21,9	29,0	46,2	67,6	87,0
$N_{Rd,p}^0$ Temperature range II [kN]	8,9	12,6	18,4	24,4	38,9	57,0	73,3
$N_{Rd,p}^0$ Temperature range III [kN]	5,6	7,9	11,5	15,3	24,3	35,6	45,8
Cracked concrete							
$N_{Rd,p}^0$ Temperature range I [kN]	-	10,4	15,2	22,0	29,2	42,7	55,0
$N_{Rd,p}^0$ Temperature range II [kN]	-	8,5	13,8	18,3	26,7	42,7	55,0
$N_{Rd,p}^0$ Temperature range III [kN]	-	5,7	8,3	12,8	17,0	24,9	36,7

Design concrete cone resistance $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$

Design splitting resistance ^{a)} $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
$N_{Rd,c}^0$ Non-cracked concrete [kN]		20,1	24,0	32,4	39,2	58,8	74,6	85,4
$N_{Rd,c}^0$ Cracked concrete [kN]		-	28,7	38,8	47,1	58,8	74,6	85,4

a) Splitting resistance must only be considered for non-cracked concrete

Influencing factors

Influence of concrete strength on combined pull-out and concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_{B,p} = (f_{ck,cube}/25N/mm^2)^{0,10}$ ^{a)}	1,00	1,02	1,04	1,06	1,07	1,08	1,09

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of embedment depth on combined pull-out and concrete cone resistance

$f_{h,p} = h_{ef}/h_{ef,typ}$

Influence of concrete strength on concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{0,5}$ ^{a)}	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of edge distance ^{a)}

$c/c_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$c/c_{cr,sp}$										
$f_{1,N} = 0,7 + 0,3 \cdot c/c_{cr,N} \leq 1$	0,73	0,76	0,79	0,82	0,85	0,88	0,91	0,94	0,97	1
$f_{1,sp} = 0,7 + 0,3 \cdot c/c_{cr,sp} \leq 1$										
$f_{2,N} = 0,5 \cdot (1 + c/c_{cr,N}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{2,sp} = 0,5 \cdot (1 + c/c_{cr,sp}) \leq 1$										

a) The edge distance shall not be smaller than the minimum edge distance c_{min} . These influencing factors must be considered for every edge distance smaller than the critical edge distance.

Influence of anchor spacing ^{a)}

$s/s_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$s/s_{cr,sp}$										
$f_{3,N} = 0,5 \cdot (1 + s/s_{cr,N}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{3,sp} = 0,5 \cdot (1 + s/s_{cr,sp}) \leq 1$										

a) The anchor spacing shall not be smaller than the minimum anchor spacing s_{min} . This influencing factor must be considered for every anchor spacing.

Influence of embedment depth on concrete cone resistance

$f_{h,N} = (h_{ef}/h_{ef,typ})^{1,5}$

Influence of reinforcement

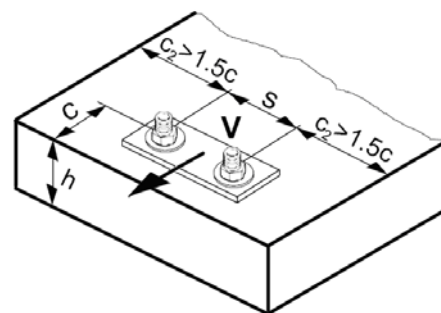
h_{ef} [mm]	40	50	60	70	80	90	≥ 100
$f_{re,N} = 0,5 + h_{ef}/200\text{mm} \leq 1$	0,7 ^{a)}	0,75 ^{a)}	0,8 ^{a)}	0,85 ^{a)}	0,9 ^{a)}	0,95 ^{a)}	1

a) This factor applies only for dense reinforcement. If in the area of anchorage there is reinforcement with a spacing ≥ 150 mm (any diameter) or with a diameter ≤ 10 mm and a spacing ≥ 100 mm, then a factor $f_{re,N} = 1$ may be applied.

SHEAR loading

The design shear resistance is the lower value of

- Steel resistance: $V_{Rd,s}$
- Concrete pryout resistance: $V_{Rd,cp} = k \cdot \text{lower value of } N_{Rd,p} \text{ and } N_{Rd,c}$
- Concrete edge resistance: $V_{Rd,c} = V^0_{Rd,c} \cdot f_B \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$



Basic design shear resistance

Design steel resistance $V_{Rd,s}$

Anchor size	$\emptyset 8$	$\emptyset 10$	$\emptyset 12$	$\emptyset 14$	$\emptyset 16$	$\emptyset 20$	$\emptyset 25$
$V_{Rd,s}$ Rebar BSt 500 S [kN]	9,3	14,7	20,7	28,0	36,7	57,3	90,0

Design concrete pryout resistance $V_{Rd,cp} = \text{lower value}^a)$ of $k \cdot N_{Rd,p}$ and $k \cdot N_{Rd,c}$

$$k = 2 \text{ for } h_{ef} \geq 60 \text{ mm}$$

- a) $N_{Rd,p}$: Design combined pull-out and concrete cone resistance
 $N_{Rd,c}$: Design concrete cone resistance

Design concrete edge resistance $V_{Rd,c} = V^0_{Rd,c} \cdot f_B \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$

Anchor size	$\emptyset 8$	$\emptyset 10$	$\emptyset 12$	$\emptyset 14$	$\emptyset 16$	$\emptyset 20$	$\emptyset 25$
Non-cracked concrete							
$V^0_{Rd,c}$ [kN]	5,9	8,6	11,6	15,0	18,7	27,0	39,2
Cracked concrete							
$V^0_{Rd,c}$ [kN]	-	6,1	8,2	10,6	13,2	19,2	27,7

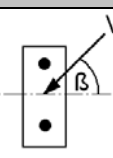
Influencing factors

Influence of concrete strength

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25\text{N/mm}^2)^{1/2}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

- a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of angle between load applied and the direction perpendicular to the free edge

Angle β	0°	10°	20°	30°	40°	50°	60°	70°	80°	≥ 90°
$f_{\beta} = \sqrt{\frac{1}{(\cos \alpha_V)^2 + \left(\frac{\sin \alpha_V}{2,5}\right)^2}}$ 	1	1,01	1,05	1,13	1,24	1,40	1,64	1,97	2,32	2,50

Influence of base material thickness

h/c	0,15	0,3	0,45	0,6	0,75	0,9	1,05	1,2	1,35	≥ 1,5
$f_h = \{h/(1,5 \cdot c)\}^{1/2} \leq 1$	0,32	0,45	0,55	0,63	0,71	0,77	0,84	0,89	0,95	1,00

Influence of anchor spacing and edge distance ^{a)} for concrete edge resistance: f_4

$$f_4 = (c/h_{ef})^{1,5} \cdot (1 + s / [3 \cdot c]) \cdot 0,5$$

c/h _{ef}	Single anchor	Group of two anchors s/h _{ef}														
		0,75	1,50	2,25	3,00	3,75	4,50	5,25	6,00	6,75	7,50	8,25	9,00	9,75	10,50	11,25
0,50	0,35	0,27	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35
0,75	0,65	0,43	0,54	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65
1,00	1,00	0,63	0,75	0,88	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00
1,25	1,40	0,84	0,98	1,12	1,26	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40
1,50	1,84	1,07	1,22	1,38	1,53	1,68	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84
1,75	2,32	1,32	1,49	1,65	1,82	1,98	2,15	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32
2,00	2,83	1,59	1,77	1,94	2,12	2,30	2,47	2,65	2,83	2,83	2,83	2,83	2,83	2,83	2,83	2,83
2,25	3,38	1,88	2,06	2,25	2,44	2,63	2,81	3,00	3,19	3,38	3,38	3,38	3,38	3,38	3,38	3,38
2,50	3,95	2,17	2,37	2,57	2,77	2,96	3,16	3,36	3,56	3,76	3,95	3,95	3,95	3,95	3,95	3,95
2,75	4,56	2,49	2,69	2,90	3,11	3,32	3,52	3,73	3,94	4,15	4,35	4,56	4,56	4,56	4,56	4,56
3,00	5,20	2,81	3,03	3,25	3,46	3,68	3,90	4,11	4,33	4,55	4,76	4,98	5,20	5,20	5,20	5,20
3,25	5,86	3,15	3,38	3,61	3,83	4,06	4,28	4,51	4,73	4,96	5,18	5,41	5,63	5,86	5,86	5,86
3,50	6,55	3,51	3,74	3,98	4,21	4,44	4,68	4,91	5,14	5,38	5,61	5,85	6,08	6,31	6,55	6,55
3,75	7,26	3,87	4,12	4,36	4,60	4,84	5,08	5,33	5,57	5,81	6,05	6,29	6,54	6,78	7,02	7,26
4,00	8,00	4,25	4,50	4,75	5,00	5,25	5,50	5,75	6,00	6,25	6,50	6,75	7,00	7,25	7,50	7,75
4,25	8,76	4,64	4,90	5,15	5,41	5,67	5,93	6,18	6,44	6,70	6,96	7,22	7,47	7,73	7,99	8,25
4,50	9,55	5,04	5,30	5,57	5,83	6,10	6,36	6,63	6,89	7,16	7,42	7,69	7,95	8,22	8,49	8,75
4,75	10,35	5,45	5,72	5,99	6,27	6,54	6,81	7,08	7,36	7,63	7,90	8,17	8,45	8,72	8,99	9,26
5,00	11,18	5,87	6,15	6,43	6,71	6,99	7,27	7,55	7,83	8,11	8,39	8,66	8,94	9,22	9,50	9,78
5,25	12,03	6,30	6,59	6,87	7,16	7,45	7,73	8,02	8,31	8,59	8,88	9,17	9,45	9,74	10,02	10,31
5,50	12,90	6,74	7,04	7,33	7,62	7,92	8,21	8,50	8,79	9,09	9,38	9,67	9,97	10,26	10,55	10,85

a) The anchor spacing and the edge distance shall not be smaller than the minimum anchor spacing s_{min} and the minimum edge distance c_{min} .

Influence of embedment depth

h _{ef} /d	4	4,5	5	6	7	8	9	10	11
$f_{hef} = 0,05 \cdot (h_{ef} / d)^{1,68}$	0,51	0,63	0,75	1,01	1,31	1,64	2,00	2,39	2,81

h _{ef} /d	12	13	14	15	16	17	18	19	20
$f_{hef} = 0,05 \cdot (h_{ef} / d)^{1,68}$	3,25	3,72	4,21	4,73	5,27	5,84	6,42	7,04	7,67

Influence of edge distance ^{a)}

c/d	4	6	8	10	15	20	30	40
$f_c = (d / c)^{0,19}$	0,77	0,71	0,67	0,65	0,60	0,57	0,52	0,50

a) The edge distance shall not be smaller than the minimum edge distance c_{min} .

Combined TENSION and SHEAR loading

For combined tension and shear loading see section “Anchor Design”.

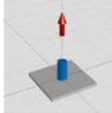
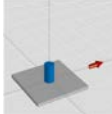
Precalculated values – design resistance values

All data applies to:

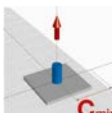
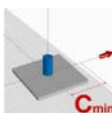
- non-cracked concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$
- temperature range I (see service temperature range)
- minimum thickness of base material
- no effects of dense reinforcement

Recommended loads can be calculated by dividing the design resistance by an overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

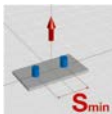
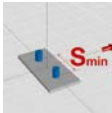
Design resistance: concrete C 20/25 - minimum embedment depth

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = h_{ef,min}$ [mm]		60	60	70	80	90	100	110
Base material thickness $h = h_{min}$ [mm]		100	100	102	116	130	150	174
	Tensile N_{Rd}: single anchor, no edge effects							
	Non-cracked concrete							
	BSt 500 S [kN]	8,0	9,9	13,9	18,6	28,7	33,7	32,4
	Cracked concrete							
BSt 500 S [kN]	-	6,9	9,7	14,1	18,1	24,0	23,1	
	Shear V_{Rd}: single anchor, no edge effects, without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	9,3	14,7	20,7	28,0	36,7	57,3	64,7
	Cracked concrete							
BSt 500 S [kN]	-	13,8	19,4	28,0	36,2	48,0	46,1	

Design resistance: concrete C 20/25 - minimum embedment depth

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = h_{ef,min}$ [mm]		60	60	70	80	90	100	110
Base material thickness $h = h_{min}$ [mm]		100	100	102	116	130	150	174
Edge distance $c = c_{min}$ [mm]		40	50	60	80	100	120	135
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)							
	Non-cracked concrete							
	BSt 500 S [kN]	4,8	6,7	9,5	12,8	19,4	24,4	25,0
	Cracked concrete							
BSt 500 S [kN]	-	4,7	6,6	10,6	14,5	20,3	19,8	
	Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	3,5	4,9	6,6	10,0	13,2	17,4	21,8
	Cracked concrete							
BSt 500 S [kN]	-	3,5	4,7	7,1	9,4	12,3	15,4	

Design resistance: concrete C 20/25 - minimum embedment depth (load values are valid for single anchor)

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = h_{ef,min}$ [mm]		60	60	70	80	90	100	110
Base material thickness $h = h_{min}$ [mm]		100	100	100	116	138	156	170
Spacing $s = s_{min}$ [mm]		40	50	60	80	100	120	135
	Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)							
	Non-cracked concrete							
	BSt 500 S [kN]	5,4	6,8	9,3	12,2	17,6	21,3	22,5
	Cracked concrete							
BSt 500 S [kN]	-	4,9	6,7	9,5	12,1	15,2	16,0	
	Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$), without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	9,3	12,7	17,9	24,0	36,7	44,9	47,1
	Cracked concrete							
BSt 500 S [kN]	-	8,8	12,4	18,2	23,5	32,0	33,6	

Design resistance: concrete C 20/25 - typical embedment depth

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]		80	90	110	125	145	170	210
Base material thickness $h = h_{min}$ [mm]		110	120	142	161	185	220	274
	Tensile N_{Rd}: single anchor, no edge effects							
	Non-cracked concrete							
	BSt 500 S [kN]	10,6	14,9	21,9	29,0	46,2	67,6	85,4
	Cracked concrete							
BSt 500 S [kN]	-	10,4	15,2	22,0	29,2	42,7	55,0	
	Shear V_{Rd}: single anchor, no edge effects, without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	9,3	14,7	20,7	28,0	36,7	57,3	90,0
	Cracked concrete							
BSt 500 S [kN]	-	14,7	20,7	28,0	36,7	57,3	90,0	

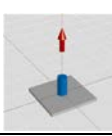
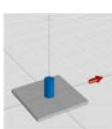
Design resistance: concrete C 20/25 - typical embedment depth

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]		80	90	110	125	145	170	210
Base material thickness $h = h_{min}$ [mm]		110	120	142	161	185	220	274
Edge distance $c = c_{min}$ [mm]		40	50	60	80	100	120	135
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)							
	Non-cracked concrete							
	BSt 500 S [kN]	6,4	9,0	13,2	18,6	30,4	38,9	43,1
	Cracked concrete							
BSt 500 S [kN]	-	6,2	9,1	14,1	19,6	28,2	34,3	
	Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	3,7	5,3	7,3	11,2	15,8	21,5	27,5
	Cracked concrete							
BSt 500 S [kN]	-	3,8	5,2	7,9	11,2	15,2	19,5	

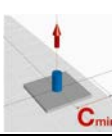
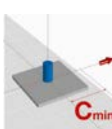
Design resistance concrete C 20/25 - typical embedment depth (load values are valid for single anchor)

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]		80	90	110	125	145	170	210
Base material thickness $h = h_{min}$ [mm]		110	120	142	161	185	220	274
Spacing $s = s_{min}$ [mm]		40	50	60	80	100	120	135
	Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)							
	Non-cracked concrete							
	BSt 500 S [kN]	7,4	10,1	14,7	19,1	30,1	42,2	49,5
	Cracked concrete							
BSt 500 S [kN]	-	7,2	10,5	14,8	19,5	27,7	35,3	
	Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$), without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	9,3	14,7	20,7	28,0	36,7	57,3	90,0
	Cracked concrete							
BSt 500 S [kN]	-	12,3	18,0	26,1	34,5	51,1	68,1	

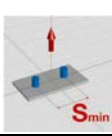

Design resistance: concrete C 20/25 - embedment depth = 12 d^{a)}

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = 12 d^{a)}$ [mm]		96	120	144	168	192	240	300
Base material thickness $h = h_{min}$ [mm]		126	150	176	204	232	290	364
	Tensile N_{Rd}: single anchor, no edge effects							
	Non-cracked concrete							
	BSt 500 S [kN]	12,7	19,9	28,7	39,0	61,1	95,5	124,4
	Cracked concrete							
BSt 500 S [kN]	-	13,8	19,9	29,6	38,6	60,3	78,5	
	Shear V_{Rd}: single anchor, no edge effects, without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	9,3	14,7	20,7	28,0	36,7	57,3	90,0
	Cracked concrete							
BSt 500 S [kN]	-	14,7	20,7	28,0	36,7	57,3	90,0	

Design resistance: concrete C 20/25 - embedment depth = 12 d^{a)}

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = 12 d^{a)}$ [mm]		96	120	144	168	192	240	300
Base material thickness $h = h_{min}$ [mm]		126	150	176	204	232	290	364
Edge distance $c = c_{min}$ [mm]		40	50	60	80	100	120	135
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)							
	Non-cracked concrete							
	BSt 500 S [kN]	7,7	12,0	17,2	25,1	41,2	58,6	66,4
	Cracked concrete							
BSt 500 S [kN]	-	8,3	12,0	19,0	26,0	39,8	49,0	
	Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	3,9	5,7	7,8	12,0	16,9	23,6	30,5
	Cracked concrete							
BSt 500 S [kN]	-	4,0	5,5	8,5	12,0	16,7	21,6	

Design resistance: concrete C 20/25 - embedment depth = 12 d^{a)} (load values are valid for single anchor)

Anchor size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Embedment depth $h_{ef} = 12 d^{a)}$ [mm]		96	120	144	168	192	240	300
Base material thickness $h = h_{min}$ [mm]		126	150	176	204	232	290	364
Spacing $s = s_{min}$ [mm]		40	50	60	80	100	120	135
	Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)							
	Non-cracked concrete							
	BSt 500 S [kN]	8,9	13,8	19,6	26,4	40,9	62,6	81,0
	Cracked concrete							
BSt 500 S [kN]	-	9,8	13,9	20,3	26,3	40,5	53,2	
	Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$), without lever arm							
	Non-cracked concrete							
	BSt 500 S [kN]	9,3	14,7	20,7	28,0	36,7	57,3	90,0
	Cracked concrete							
BSt 500 S [kN]	-	14,7	20,7	28,0	36,7	57,3	90,0	

a) d = element diameter